

EngiLab Beam.2D 2015 v2.2

(v2.2.5602.0)

User Manual



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EngiLab Beam.2D 2015 v2.2 User Manual

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Document release date October 3, 2017 "I was originally supposed to become an engineer, but the thought of having to expend my creative energy on things that make practical everyday life even more refined, with a loathsome capital gain as the goal, was unbearable to me"

Albert Einstein



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Chapter

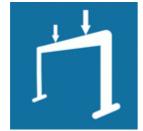


Introduction to EngiLab Beam.2D

1 Introduction to EngiLab Beam.2D

- Overview
- Lite and Pro Editions

1.1 Overview



EngiLab Beam.2D 2015

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EngiLab Beam.2D is an easy-to-use yet powerful engineering tool for the linear static analysis of plane (2D) structures for Windows. It can analyze **continuous beams**, **plane frames** and **plane trusses**. The program features a full Graphical User Interface (GUI) for pre-processing or post-processing and uses the **Finite Element Method (FEM)** for its analysis purposes.

Communication

For general inquiries, please contact us at info@engilab.com

Technical support

EngiLab offers technical support via email at support@engilab.com. Email questions are normally answered within 24 hours for registered users. Considering our customer support team's busy work schedule, EngiLab strives to provide technical support, via email, to non-registered (Lite Edition) users as well.

Required Information for Support

When sending us a question via email, please make sure to include the following information:

- Operating system (Windows XP, Windows Vista, Windows 7, Windows 8, etc.)
- Name and exact version of the product being used.
- Brief description of the problem.
- Detailed instructions on how to duplicate the problem, and/or an input file as an attachment.

You may find additional information, new releases, etc on the web at <u>www.engilab.com</u>. We take customer satisfaction very seriously and always welcome your feedback.



Compatibility

EngiLab Beam.2D is compatible with the following versions of Windows (32 bit and 64 bit):

- Windows XP, Windows 2000, Windows Vista, Windows 7, Windows 8, Windows 8.1
- Windows Server 2003, Windows Server 2008, Windows Server 2008 R2, Windows Server 2012



Limited Warranty - Disclaimer of Damages

See the <u>License Agreement</u>.

1.2 Lite and Pro Editions

EngiLab Beam.2D comes in two Editions: **Lite** and **Pro**. The Lite Edition is completely free, but it has limitations compared to the full Pro Edition, as described in detail below.

Lite Edition

- Free of charge, available for download at www.engilab.com
- Only for **non-profit use** (personal, academic or educational purposes). It may *not* be used for any commercial purposes.
- Analysis limitations: The Lite Edition can open, save and modify any structural Model. Its input files are exactly the same as the ones of the Pro Edition. Yet, the Lite Edition can only analyze small Models of up to 10 Nodes, 3 Materials and 3 Sections.
- **No missing features**: You get the real experience of the Pro Edition. All the functions of the Pro Edition are present also in the Lite Edition. No trial version banners printed anywhere, no annoying pop-ups, no hidden fees.
- Ideal for: Students and Hobbyists who need to analyze small models, or Professional Engineers who would like to test the Program before purchasing the Pro Edition.
- Technical Support for the Lite Edition is only provided via email at support@engilab.com



Pro Edition

- The Pro Edition is available for purchase at <u>www.engilab.com</u>.
- It can be used for **any purpose** (personal, academic, educational or **commercial use**)
- **No modeling limitations**: The Pro Edition can analyze any structural model (Unlimited number of Nodes, Materials and Sections).
- Ideal for: Professional Engineers, Universities, Students, Researchers, Hobbyists who wish to analyze models without any limitations.
- **Priority Technical Support** is provided within 24 hours via email at <u>support@engilab.com</u>, or by phone.



Chapter



Structural Modeling

2 Structural Modeling

- System of Units
- Setting up a Model
- Model Properties
 - <u>Materials</u>
 - <u>Sections</u>
 - <u>Nodes</u>
 - Elements
 - <u>Nodal Loads</u>
 - <u>Elemental Loads</u>
 - Body (Acceleration) Loads

2.1 System of Units

EngiLab Beam.2D has no default system of units. This is not a limitation of the program, but a deliberate choice in order for the program to work globally, no matter what system of units is used. This way there is no limitation in the system of units that can be used. **Any consistent system of units can be used**.

Before starting to define any model, you need to decide which system of units you will use. All input data must be specified in **consistent units**. As a result, the analysis results will also comply to that system. The important point about using consistent units is the necessity to stick with units that work correctly together - not to mix units that do not have a correct relationship with each other.

In order to define a consistent system of units, you have to define first the **primary** (**basic**) units and then the **derived units** which are dependent on the primary units. We propose two different approaches for defining a consistent system of units, as described in detail below:



A. Consistent system of units based on Force

Define the three primary (basic) units for Force (F), Length (L), Time (T). For example you can choose to use kN, m, s (sec). The **derived units** are then the following:

Derived Unit	Formula	Formula In our example (kN explanation m, s)
Acceleration	L/T ²	(1 Length unit) / (1 _{m/s²} time unit) ²
Mass *	F·T ² /L	(1 force unit) / (1kN/(m/s ²) = Mg = t * acceleration unit)
Density	F·T ² /L ⁴	(1 mass unit) / (1t/m³ length unit) ³
Stress	F/L ²	(1 force unit) / (1kN/m ² length unit) ²

* The mass unit (in our example 1 t) is the mass that accelerates by the acceleration unit rate (in our example 1 m/s²) when the unit force (in our example 1 kN) is exerted on it.

B. Consistent system of units based on Mass

Define the three primary (basic) units for **Mass (M), Length (L), Time (T)**. For example you can choose to use kg, m, s (sec). The **derived units** are then the following:

Derived Unit	Formula		In our example (kg, m, s)
Acceleration	L/T ²	(1 Length unit) / (1 time unit) ²	m/s²
Force *	M·L/T ²	(1 mass unit) · (1 acceleration unit)	$kg \cdot m/s^2 = N *$
Density	M/L ³	(1 mass unit) / (1 length unit) ³	kg/m ³
Stress	M/L/T ²	(1 force unit) / (1 length unit) ²	N/m ²

* The force unit (in our example 1 N) is the force required to accelerate the unit mass (in our example 1 kg) at the acceleration unit rate (in our example 1 m/s²).



Common consistent systems of units

Some common systems of consistent units are shown in the table below.

Quantity	SI (MKS)	MTS	mmNS	US Unit (ft)	US Unit (in)		
Length	m	m	mm	ft	in		
Force	N	kN	N	lbf	lbf		
Mass	kg	t (tonne)	t	slug	lbf∙s²/in		
Time	S	S	s	S	s		
Stress	Pa (N/m ²)	kPa (kN/m ²)	MPa	lbf/ft ²	psi (lbf/in ²)		
Density	kg/m ³	t/m ³	t/mm³	slug/ft ³	lbf∙s²/in⁴		
Acceleration	m/s²	m/s ²	mm/s ²	ft/s ²	in/s ²		

As points of reference, the mass density of steel, the Young's Modulus of steel and the standard earth gravitational acceleration are given for each system in the table below.

Quantity	SI (MKS)	MTS	mmNS	US Unit (ft)	US Unit (in)
Steel density	7850 kg/m ³		mm ³	slug/ft ³ (lbf·s²/ft ⁴)	7.34544493×1 0 ⁻⁴ blob/in ³ (lbf·s²/in ⁴)
Steel E	210×10 ⁹ N/ m ²	210×10 ⁶ kN/ m ²	210×10 ³ MPa	4,385,941,188 .96153 lbf/ft ²	30,457,924.92 33 psi
Earth Gravity	9.80665 m/s ²				386.088583 in/s ²

Notes:

- 1 t (tonne) = 10^3 kg = 1 Mg. It is a mass that accelerates by 1 m/s² when a force of 1 kN is exerted on it.
- 1 slug = 1 lbf·s²/ft. It is a mass that accelerates by 1 ft/s² when a force of 1 pound-force (lbf) is exerted on it.
- 1 blob = 1 lbf·s²/in. It is a mass that accelerates by 1 in/s² when a force of 1 pound-force (lbf) is exerted on it.
- 1 MPa = 1 MN/ m^2 = 1 N/ mm^2

Practical example

The user chooses to use the **MTS system**, a common choice for structural engineering applications:

- Length: **m**
- Force: **kN**
- Time: s



EngiLab Beam.2D data have to be given as shown below:

Quantity	Unit used
Node Coordinates X, Y	m
Material Elastic Modulus E	$kPa = kN/m^2$
Material Density d	t/m³
Section Area A	m ²
Section Moment of Inertia I	m ⁴
Nodal Force F	kN
Nodal Moment M	kN∙m
Elemental load f	kN/m
Spring Elastic Stiffness KX, KY	kN/m
Spring Elastic constant KZ	kN∙m (/RAD) *
Acceleration	m/s²

The results will also comply to that system, thus they will be given as:

Quantity	Unit used
Node X, Y Displacement	m
Node Z Rotation	RAD *
Axial, Shear Force at Element end i, j	kN
Moment at Element end i, j	kN∙m
Support reaction FX, FY	kN
Support reaction MZ	kN∙m

* Rotations are ALWAYS given in RADIANS.

Note: In the above example, if one wants to apply self-weight to the structure, he can add the standard earth gravitational acceleration at the -y direction: $\mathbf{ay} = -9.80665$



2.2 Setting up a model

The user can set-up, analyze and view the results of a model using the following **12** simple steps:

A. Preparation of the input data

1. Define the **System of Units**. This is a very important step for the model. This step has nothing to do with the program itself. Instead, the user has to define the preferred system of units and then all the program data have to be consistent with this system. Then the results will also comply to that system. For details, see <u>System of Units</u>.

B. Pre-Processing: Building the Model

_		TOOLS						
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2. Click $\mathbb{Z}_{\underline{E}}$ to define **Material**(s). You need to define the Material **Name** (optional), Material **Density d** (optional) and Material **Elastic Modulus E** for each Material. For details, see <u>Materials</u>.

3. Click \mathbf{I} to define **Sections**(s). You need to define the Section **Name** (optional), **Section Area A** and Section **Moment of Inertia I** for each Section. For details, see <u>Sections</u>.

4. After you have defined at least one Material and one Section, you can **start drawing your Model on screen**, as follows:

- Left-click on screen to define a new free Node (no constraints).
- Left-click on screen, hold down the left button and then release it at another location to define a new Element and two nodes at ends i, j.
- Double-click on a Node or close to it to to define a new Constraint (Support) set. Each time you double-click a Node, a new Nodal Constraint set is applied to the Node.
- If **SNAP** is activated (Default=True), then you can only draw Nodes and Elements at increments defined by the Snap Size setting (Default=0.1).
- If **SNAPNODE** is activated (Default=True), then you can "catch" Nodes so that new elements can be connected to existing Nodes.
- Note: All Elements that are defined on screen are assigned Material 1 and Section 1.

If you do not want to use on-screen drawing, you can still click * to define Nodes manually and \checkmark to define Elements manually.

5. Click * to:



- Move **Nodes** to their exact positions, if needed. For example, a Node with X-Coordinate 5.8 defined on screen with Snap Size = 0.1 should be moved to the exact position 5.85.
- Define or change Nodal Constraints.
- Define Springs.

For details, see <u>Nodes</u>.

- **6**. Click 🖊 to:
- Assign the right Material and Section to every **Element**, if needed. All Elements that are defined on screen are assigned Material 1 and Section 1. For example, an Element that has been defined on screen has to be assigned Material 2 or Section 2.
- Define Hinges at Element ends. Each Element can have a hinge at either end (Start-i or End-j or both).

For details, see <u>Elements</u>.

- 7. Click to define **Nodal Loads**. For details, see <u>Nodal Loads</u>.
- **8**. Click III to define **Elemental Loads**. For details, see <u>Elemental Loads</u>.

9. Click **9** to define **Body (Acceleration) Loads** (if needed). For details, see <u>Body</u> (Acceleration) Loads.

- If you want to take into account the self-weight of Elements as an additional elemental load for each Element, then you have to provide the Material Density for the Material of each Element and also to define a Linear Acceleration Vector equal to the standard earth gravitational acceleration. A common practice is to put the earth gravitational acceleration with a minus (-) sign at the Y direction this means gravity acting towards -Y global axis.
- Example: If you are using kN for forces, m for length and s for time, then the Material Density has to be given in t/m³ and you have to enter -9.80665 (or simply -9.81) at the aY component of the Linear Acceleration Vector.

C. Analyze the Model



10. Click (or press F5) to **run the Finite Element Analysis**. For details on the Analysis, see <u>Analysis</u>.



D. Post-Processing: See the Analysis Results

 EILE
 MODEL
 TOOLS
 VIEW
 HELP
 Post-Processing Tools

 Image: I

11. Click N, V, M, \checkmark or F to see the **Axial Force Diagram**, **Shear Force Diagram**, **Bending Moment Diagram**, **Model Deformation** or the **Free Body Diagram**.

- N: <u>Axial Force Diagram</u>
- V: Shear Force Diagram
- M: Bending Moment Diagram
- 📥: Deformation (deformed shape of the Model)
- F: Free Body Diagram (external forces together with support reactions)

12. Click **I** to see the **analytical results**. The analytical results include:

- <u>Node displacements</u>
- Element forces
- Support reactions
- Full Analysis Report in Rich Text Format (RTF)
- Analysis validation

2.3 Model Properties

- Materials
- Sections
- <u>Nodes</u>
- Elements
- Nodal Loads
- Elemental Loads
- Body (Acceleration) Loads

2.3.1 Materials

<u>F</u> ILE	MODEL	TOOLS	VIEW	HELP									
ኈ 省	2	C S C	t 🔍 🔇	R @ <u>%</u> I	•	Z 1 ±	<u>m</u> g	۲	Ν	V	М¥	F	Ē

Each element is assigned a Material. The Material properties are the following:

- Name (optional, max. number of characters in Name: 20)
- **Density d** (optional, can be omitted, see below)



• Elastic Modulus E

The **Elastic Modulus E** of the Material is an important parameter affecting the Model Stiffness Matrix.

The **Density** is used in order for the program to calculate the Body Loads due to Linear Acceleration. For example, if you want to take into account the self-weight of Elements as an additional elemental load for each Element, then you have to provide the Material Density for the Material of each Element and also to define a Linear Acceleration Vector equal to the standard earth gravitational acceleration. If Density is ommitted or it is equal to zero for a Material, then Elements that are assigned this Material will not take any Body (Acceleration) Loads even if a Linear Acceleration Vector is defined. By setting Density equal to zero for a Material, you can model Weightless Elements.

<u>ν</u>		Materials		- 5	×			
Mat. ID	Name	Density d	El. Modulus E					
▶ 1	Steel	7.85	21000000					
Add new l	Material Actions for se	elected Selection t	ools					
New Mat	New Material properties							
Name	d =	Add	new Material					
	E =				ОК:			

See also: Body (Acceleration) Loads.

Add new Material

To add a new Material, type the Name (optional), Density (optional) and Elastic Modulus E of the Material and click the "Add new Material" button.

Edit an existing Material

You can click on the table and you can easily edit the properties of an already existing



Material. Any change you make is automatically reflected to the Model.

Actions for selected Materials

In order to perform these actions, you have to first select the entire row(s) of the corresponding Material(s) and then apply the action.

- **Duplicate Material**(s). Click the "Duplicate" button. You can Duplicate more than one Materials at a time.
- **Delete Material**(s). Click the "Delete" button. You can Delete more than one Materials at a time.

Σ <u>ε</u>		Materials		_ 1	×
Mat. ID	Name	Density d	El. Modulus E		
▶ 1	Steel	7.85	21000000		
Add new 1	Material Actions for se	elected Selection t	ools		
Duplicate		Belection	0013	0	
		elete			
Dup		iete			ОК
					.::



Materials selection tools

This tab provides tools for the selection of Materials. For example, you can select (add to selection) or deselect (remove from selection) all the Materials that are assigned to Elements, or all the Materials that are NOT assigned to Elements.

Žε.		Mater	ials	- 🗆 🗙
Mat. ID	Name	Density d	El. Modulus l	E
▶ 1	Steel	7.85	21000000	
Add new l	Material Actions for	selected Selec	tion tools	
Add to se	lection / Remove fro	m selection	Select all	0
Assig	A00 (+)			
⊖ Not a	ssigned	Remove (-)	Clear all	ОК

2.3.2 Sections

Each element is assigned a **Section**. The Section properties are the following:

- Name (optional, max. number of characters in Name: 20)
- Area A
- Moment of Inertia I

The **Area A** and the **Moment of Inertia I** of the Section are important parameters affecting the Model Stiffness Matrix.





I		Sections			×
Sec. ID	Name	Area A	Mom. of Inertia I		
▶ 1	Section1	0.005	0.0025		
Add new 9	Section Actions for se	lected Selection to	ols		
	ion properties		0		
Name	A =		Calculator		
	I =	Add	new Section	ОК	

Add new Section

To add a new Section, type the Name (optional), Area A and Moment of Inertia I of the Section and click the "Add new Section" button.

Edit an existing Section

You can click on the table and you can easily edit the properties of an already existing Section. Any change you make is automatically reflected to the Model.



Section Properties Calculator

By clicking the "**Calculator**" button, a new form appears, as shown below. You can use the Section Properties Calculator to calculate the properties of various section shapes. The various section shapes are shown in the right Picture below. The Calculator calculates various properties. The program needs only to take the Area A and Moment of Inertia I property. Click "Apply y-y" to apply the Area A and the Iy Moment of Inertia. Click "Apply z-z" to apply the Area A and the Iz Moment of Inertia.

D.	Section P	ropertie	s Calculator	- 🗆 🗙
Section Type: Wide flange (No	rmal) 🗸	Prop.	Details	Value
		уС	Centroid y	0.15
t ^z d	a = 0.3	zC	Centroid z	0.3
	b = 0.6	Α	Area	0.00831
v k	c = 0.005	ly	Mom. of Inertia y	0.0005537064
y the y	d = 0.009	lz	Mom. of Inertia z	4.05061E-05
		yPNA	Plastic Neutral axi	0.15
←a		zPNA	Plastic Neutral axi	0.3
2		Wply	Plastic Modulus y	0.002019105
	Decimals 10 💂	Wplz	Plastic Modulus z	0.0004086375
		Apply y	-y Apply z-z	Cancel

The available sections are the following:

- Square bar
 Square tube
 Rectangular bar
 Rectangular tube
 Circular bar
 Circular tube
 Circular tube
 Cross (Normal)
 Cross (General)
 Wide flange (Normal)
 Wide flange (General)
 T-Shape
 C-Shape (General)
 C-Shape (General)
- L-Shape (Normal)
- L-Shape (General)



Actions for selected Sections

In order to perform these actions, you have to first select the entire row(s) of the corresponding Section(s) and then apply the action.

- **Duplicate Section**(s). Click the "Duplicate" button. You can Duplicate more than one Sections at a time.
- **Delete Section**(s). Click the "Delete" button. You can Delete more than one Sections at a time.

I		Sec	tions		×
Sec. ID	Name	Area A	Mom. of In	ertia I	
▶ 1	Section1	0.005	0.0025		
Add new 9	Section Actio	ons for selected Sele	ction tools		
Duplicate		Delete		•	
Dup	licate	Delete			
				OK	



Sections selection tools

This tab provides tools for the selection of Sections. For example, you can select (add to selection) or deselect (remove from selection) all the Sections that are assigned to Elements, or all the Sections that are NOT assigned to Elements.

I		Sections		-		x
Sec. ID	Name	Area A	Mom. of Inertia I			
▶ 1	Section1	0.005	0.0025			
		lected Selection to	ols			
Add new !						
Add to se	lection / Remove from	selection	Select all	0		
Assig	ned ssigned Add (+)	Remove (-)	Clear all		OK	
	ssigned		Clear all		UK.	

2.3.3 Nodes

Nodes connect Elements. Each Element has a Start Node (i) and an End Node (j). Each node is located at a (X, Y) Point (Global Axes) in the 2D plane and it has 3 Degrees Of Freedom (DOFs):

- 🗄 X-Displacement
- + Y-Displacement
- 🛱 Z-Rotation

Each DOF can be:

- Free to move (No support = No , No spring)
- Fixed (Constrained, not able to move at all)
- With spring on (Given the spring stiffness, the spring provides a reaction force that is proportional to the deflection of the corresponding DOF)



Node properties

The Node properties are the following:

- Coordinates: X, Y
- Constraints: DX-Con, DY-Con, RZ-Con (Each checked or Not)
- Springs stiffness: DX-Stiff, DY-Stiff, RZ-Stiff

٠				No	odes			-	□ ×	
Node ID	x	Y	DX-Con	DY-Con	RZ-Con	DX-Stiff	DY-Stiff	RZ-Stiff		
▶ 1	0	0	✓	✓	✓	0	0	0		
2	0	3				0	0	0		
3	5	3				0	0	0		
4	5	0	✓	✓	✓	0	0	0		
5	10	3				0	0	0		
6	10	0	✓	✓	✓	0	0	0		
7	0	6				0	0	0		
8	5	6				0	0	0		
9	10	6				0	0	0		
Add new Node Actions for selected Selection tools New Node coordinates Image: Coordinates X= Add new Node OK										

Coordinates

Each node is located at a (X, Y) Point (Global Axes) in the 2D plane.

Constraints

The properties **DX-Con**, **DY-Con**, **RZ-Con** determine whether the corresponding DOF of the Node is constrained or not.

Each DOF can be:

- Free (0 Unchecked), or
- Fully constrained (1 Checked).



<u>Picture</u>	DX-Constraint	DY-Constraint	<u>RZ-Constraint</u>	Description
	NO	NO	NO	Free Node (000)
	\checkmark	NO	NO	y-Roller (100)
Δ	NO	\checkmark	NO	x-Roller (010)
$\stackrel{\dots}{\longrightarrow}$	\checkmark	\checkmark	NO	Pinned (110)
TITI	\checkmark	\checkmark	\checkmark	Fixed (111)
	NO	NO	\checkmark	(001)
500	\checkmark	NO	\checkmark	(101)
,,,,,,,	NO	\checkmark	\checkmark	(011)

As a result, there are in total 8 types of Nodes:

Springs

The properties **DX-Stiff**, **DY-Stiff**, **RZ-Stiff** determine the Stiffness of the Spring (Elastic constant) of the corresponding DOF. A spring provides a spring reaction force that is proportional to the deflection at the corresponding DOF.

Note: A DOF that has a spring should not be constrained, as there is no point in that. In any case, if for a given DOF there is a spring and it is also constrained, then the spring is ignored and the DOF is handled as constrained.

Add new Node

To add a new Node, type the X and Y Coordinates of the Node and click the "Add new Node" button.

Edit an existing Node

You can click on the table and you can easily edit the properties of an already existing Node. Any change you make is automatically reflected to the Model.



Actions for selected Nodes

In order to perform these actions, you have to first select the entire row(s) of the corresponding Nodes(s) and then apply the action.

- **Set Constraints**: Select the proper constraints and then click the "Set" button. You can Set Constraints for more than one Nodes at a time.
- Move or Copy Node(s): Give the Move/Copy Vector {V}={dX, dY}, select "Move" or "Copy" and then click the "Apply" button. You can Move or Copy more than one Nodes at a time.
- **Delete Node(s)**: Click the "Delete" button. You can Delete more than one Nodes at a time.

٠					No	odes			-	□ ×	
Ν	lode ID	х	Y	DX-Con	DY-Con	RZ-Con	DX-Stiff	DY-Stiff	RZ-Stiff		
	1	0	0	✓	✓	✓	0	0	0		
	2	0	3				0	0	0		
	3	5	3				0	0	0		
	4	5	0	•	•	✓	0	0	0		
	5	10	3				0	0	0		
	6	10	0	✓	✓	✓	0	0	0		
	7	0	6				0	0	0		
	8	5	6				0	0	0		
	9	10	6				0	0	0		
	Add new Node Actions for selected Selection tools Set Constraints Move / Copy Move Delete DX-Constraint • Set Copy Move DY-Constraint • Set dX= Copy Delete OK RZ-Constraint • Set dY= Apply Delete OK										



Nodes selection tools

This tab provides tools for the selection of Nodes. For example, you can select (add to selection) or deselect (remove from selection) all the Nodes that are connected to Elements, or all the Nodes that are NOT connected to Elements. This is useful when for example you need to Delete all the Nodes of the Model that are not connected to Elements.

•				No	odes			-	
Node ID	x	Y	DX-Con	DY-Con	RZ-Con	DX-Stiff	DY-Stiff	RZ-Stiff	
▶ 1	0	0	✓	✓	✓	0	0	0	
2	0	3				0	0	0	
3	5	3				0	0	0	
4	5	0	✓	✓	✓	0	0	0	
5	10	3				0	0	0	
6	10	0	✓	✓	✓	0	0	0	
7	0	6				0	0	0	
8	5	6				0	0	0	
9	10	6				0	0	0	
Add new Node Actions for selected Selection tools Add to selection / Remove from selection Select all • Connected • Not connected • Add (+) Remove (-) • Clear all • OK									

2.3.4 Elements

-		TOOLS									
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An Element connects two Nodes with each other. It can have rotation releases (hinges) at each one of its ends (start i and/or end j) and it has to be assigned a Material and a Section.

The **Element** properties are the following:

- Material
- Section
- Nodes: Node i (Start Node), Node j (End Node)
- Hinges: Hinge i (Hinge at Start Node), Hinge j (Hinge at End Node), each Checked or Not Checked
- Length: Length of the Element (Read-only property, calculated automatically by the program based on the locations of the Start and End Nodes)



/		Eleme	nts			-		×	
Elem. ID	Material	Section	Node i	Node j	Hinge i	Hinge j	Length		
▶ 1	1. Concrete	1. Columns	1	2			3.00		
2	1. Concrete	2. Beams	2	3			5.00		
3	1. Concrete	1. Columns	3	4			3.00		
4	1. Concrete	2. Beams	3	5			5.00		
5	1. Concrete	1. Columns	5	6			3.00		
6	1. Concrete	1. Columns	2	7			3.00		
7	1. Concrete	2. Beams	7	8			5.00		
8	1. Concrete	1. Columns	8	3			3.00		
9	1. Concrete	2. Beams	8	9			5.00		
10	1. Concrete	1. Columns	9	5			3.00		
Add new I	Element Actions for se	lected Selection tools							
New Elem	ent properties	odei Nodej				•	•		
Material 1. Concrete ✓ Section 1. Columns ✓ Ji Hinges I									
Section	Section 1. Columns V i Hinges j								

Add new Element

To add a new Element, select the Material and the Section and type the Start and End Nodes of the new Element. You can optionally add hinges to the new Element. Then click the "Add new Element" button.

Edit an existing Element

You can click on the table and you can easily edit the properties of an already existing Element. Any change you make is automatically reflected to the Model.



Actions for selected Elements

In order to perform these actions, you have to first select the entire row(s) of the corresponding Element(s) and then apply the action.

- **Divide Element(s)**: Type the division segments and then click the "Divide" button. You can Divide more than one Elements at a time.
- **Delete Element(s)**: Click the "Delete" button. You can Delete more than one Elements at a time.

1			Eleme	nts			-		x
Ele	em. ID	Material	Section	Node i	Node j	Hinge i	Hinge j	Length	
•	1	1. Concrete	1. Columns	1	2			3.00	
	2	1. Concrete	2. Beams	2	3			5.00	
	3	1. Concrete	1. Columns	3	4			3.00	
	4	1. Concrete	2. Beams	3	5			5.00	
	5	1. Concrete	1. Columns	5	6			3.00	
	6	1. Concrete	1. Columns	2	7			3.00	
	7	1. Concrete	2. Beams	7	8			5.00	
	8	1. Concrete	1. Columns	8	3			3.00	
	9	1. Concrete	2. Beams	8	9			5.00	
	10	1. Concrete	1. Columns	9	5			3.00	
Ad	ld new l	Element Actions for se	elected Selection tools	;					
Di	ivide	De	lete					9	
Se 2	egments	Divide	Delete					OK	
									.::



Elements selection tools

This tab provides tools for the selection of Elements. For example, you can select (add to selection) or deselect (remove from selection) all the Elements that are assigned a specific Material and/or Section.

/		Ele	ements			-	
Elem. ID Material		Section	Section Node i N		Hinge i	Hinge j	Length
▶ 1	1. Concrete	1. Columns	1	2			3.00
2	1. Concrete	2. Beams	2	3			5.00
3	1. Concrete	1. Columns	3	4			3.00
4 1. Concrete		2. Beams	3	5			5.00
5 1. Concrete		1. Columns	5	6			3.00
6	1. Concrete	1. Columns	2	7			3.00
7	1. Concrete	2. Beams	7	8			5.00
8	1. Concrete	1. Columns	8	3			3.00
9	1. Concrete	2. Beams	8	9			5.00
10	1. Concrete	1. Columns	9	5			3.00
Add new Element Actions for selected Selection tools Add to selection / Remove from selection Selection Mat 1. Concrete Add (+) Sec. 1. Columns Add (+)							ОК

2.3.5 Nodal Loads

-		TOOLS											
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Point loads can only be NODAL loads (acting on Nodes). To define a load (FX, FY and/ or MZ) at a specific point, first a Node must have been defined at that location.

The **Nodal Load** properties are the Loads in each Degree Of Freedom (DOF) of the Node (Sign convention: Global axes):

- 🗄 Force FX
- 🕂 Force FY
- 🖹 Bending Moment MZ



ł			N	odal Loads			×
		Node ID	Force FX	Force FY	Moment MZ		
Þ	1	2	30				
	2	7	50				
			7				
A	Add new M	Nodal Load	Actions for selec	ted Selection to	ols		
'	New Nod	ID 🔘	Cartesian (FX, FY) Polar (F, θ)	FX= FY= MZ=	Add New	0	ОК .::

Add new Nodal Load

To add a new Nodal Load, specify the Node ID and then type the FX, FY and MZ values of the Nodal Load set. Then click the "Add Nodal Load" button.

Actions for selected Nodal Loads

In order to perform these actions, you have to first select the entire row(s) of the corresponding Nodal Load(s) and then apply the action.

• **Delete Nodal Load(s)**. Click the "Delete" button. You can Delete more than one Nodal Loads at a time.

÷			N	lodal Loads		-		×
		Node ID	Force FX	Force FY	Moment MZ			
Þ	1	2	30					
	2	7	50					
A	dd new l	Nodal Load	Actions for sele	cted Selection to	ools			
D	elete					6		
	Delet	te					ОК	



Nodal Loads selection tools

This tab provides tools for the selection of Nodal Loads. For example, you can select (add to selection) or deselect (remove from selection) all the Nodal Loads that have FX, FY and/or MZ values.

÷		N	lodal Loads		- 🗆 🗙			
	Node ID	Force FX	Force FY	Moment MZ				
▶ 1	2	30						
2	7	50						
Add new	Nodal Load	Actions for sele	cted Selection to	ools				
	_	move from select	ion	Select all	0			
	OR FX Select all Select all AND FY Add (+) Exactly MZ							

2.3.6 Elemental Loads

_		TOOLS											
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The **Elemental Load** properties are the Loads acting at the X and/or Y directions (in Global axes system) along an Element. EngiLab Beam.2D supports linear varying loads along Elements. Uniform and triangular loads can be considered as special cases of the more general linear varying load case. The Elemental Loads are applied along the Element and must be given as Force per Unit Length of the element. The user specifies the start and end value of the Elemental force, per unit length of the element.



The Elemental Load properties are the following:

Elemental Load in X-Direction

- 🗄 Force fXi: Start vale (at start i) of the X-Linear varying Load acting along the element
- 🗄 Force fXj: End vale (at end j) of the X-Linear varying Load acting along the element

Elemental Load in Y-Direction

- [+1] Force FYi: Start vale (at start i) of the Y-Linear varying Load acting along the element
- [+1] Force FYj: End vale (at end j) of the Y-Linear varying Load acting along the element

П				Elemental Loa	ads		x
		Elem. ID	Force fXi	Force fXj	Force fYi	Force fYj	
►	1	2	5	15			
	2	5			-20	-20	
		Elemental Lo	Actions for s	elected Selectio	n tools		
		iental Load	Start (i)	End (j)		0	
Ele	m. ID	Uniform	fX=	Ad	ld Elem. Load		
		Linear varyin	ig fY=			ОК	
							.::

Add new Elemental Load

To add a new Elemental Load, first specify the Element ID. Then select Uniform or Linear varying Load. In general, all Elemental Loads are considered as linear varying loads by the program, but if you select a uniform load then the program will ask only for two values fX=fXi=fXj and fY=fYi=fYj. If you select Linear varying load, then the



program will ask for all four values, fXi, fXj, fYi and fYj. After you have finished with the loads, then click the "Add Elemental Load" button.

Actions for selected Elemental Loads

In order to perform these actions, you have to first select the entire row(s) of the corresponding Elemental Load(s) and then apply the action.

• **Delete Elemental Load(s)**. Click the "Delete" button. You can Delete more than one Elemental Loads at a time.

Π			Elemental Loa	ads	- 🗆 🗙
	Elem. ID	Force fXi	Force fXj	Force fYi	Force fYj
▶ 1	2	5	15		
2	5			-20	-20
	Elemental Lo	ad Actions for s	selected Selectio	on tools	
Delete					0
De	elete				ОК



Elemental Loads selection tools

This tab provides tools for the selection of Elemental Loads. For example, you can select (add to selection) or deselect (remove from selection) all the Elemental Loads that have fX and/or fY values.

Ξ				Elemental Loa	ads		×
		Elem. ID	Force fXi	Force fXj	Force fYi	Force fYj	
•	1	2	5	15			
	2	5			-20	-20	
Add	d new l	Elemental Lo	ad Actions for s	selected Selectio	on tools		
Ad		lection / Re	move from select	ion		0	
8	OR AND	fX 🗌	Add(c)	Demous ()	Select all		
ŏ	Exactl		Add (+)	Remove (-)	Clear all	ОК	

2.3.7 Body (Acceleration) Loads

<u>F</u> ILE	MODEL	TOOLS	VIEW	HELP											
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You can specify the X and Y components of an Acceleration vector acting on the Model. If the Model has Mass (Non-zero Density is defined for the Model's Material), then the acceleration will cause inertial body (Elemental) forces on the Elements of the Model, according to Newton's Second Law of Motion. These elemental forces are calculated automatically by the program and the results are shown on the form.

To apply an acceleration vector, type the values for the aX and aY acceleration components and **press the ENTER on the keyboard**. The model is updated automatically.



٠			Body (Ac	celeration) Loa	ads	- 🗆 🗙				
Elem	. ID	Material Density	Section Area	X-Body Load	Y-Body Load	Length				
<u>ب</u>	1	2.5	0.25	0	-6.13125	3.00				
:	2	2.5	0.125	0	-3.065625	5.00				
:	3	2.5	0.25	0	-6.13125	3.00				
4	4	2.5	0.125	0	-3.065625	5.00				
	5	2.5	0.25	0	-6.13125	3.00				
	6	2.5	0.25	0	-6.13125	3.00				
1	7	2.5	0.125	0	-3.065625	5.00				
	8	2.5	0.25	0	-6.13125	3.00				
	9	2.5	0.125	0	-3.065625	5.00				
	10	2.5	0.25	0	-6.13125	3.00				
Linea		eleration Vector				0				
aX = Delete OK										

Applications

1. Define the self-weight of the Model or the self-weight of specific Elements

If you want to take into account the self-weight of Elements as an additional elemental load for each Element, then you have to provide the Material Density for the Material of each Element (See <u>Materials</u>) and also to define a Linear Acceleration Vector equal to the standard earth gravitational acceleration. A common practice is to put the earth gravitational acceleration with a minus (-) sign at the Y direction - this means gravity acting towards -Y global axis. **See also** <u>System of Units</u>.

Example: If you are using kN for forces, m for length and s for time, then the Material Density has to be given in t/m^3 and you have to input -9.80665 (or simply -9.81) at the aY component of the Linear Acceleration Vector. This way, the self-weight will be applied to all Elements of the Model that have Mass (i.e. are assigned a Material which has Density).

2. Define forces acting on a moving (accelerating) object

Acceleration is not only Gravity. If an object accelerates, then inertial forces are applied on it, provided that the object has Mass. For example, you can analyze a structure that is fixed on a moving (accelerating) vehicle. Even if there are no other loads, the acceleration of the vehicle will cause inertial forces on the object. By providing the X and Y components of the acceleration vector, you can calculate these forces and analyze the Model.



Chapter



Tools

3 Tools

- Import Materials/Sections
- <u>Convert Model to Truss</u>

3.1 Import Materials/Sections

FILE MOD	EL TOOLS	VIEW	HELP	_				
16 省 💾 🛛	🤈 🛅 Im	port Mate	rials/Sections	17 I ±	шIд	🖉 N	VМч	F 🕨 🗉
	Co	nvert Mo	del to Truss					

The user can import **Materials** and/or **Sections** from an existing EngiLab Beam.2D file (.bea file).

3 0	Import Materials/Sections from file		×
Materials Import M Ignore d	laterials uplicate Materials		
Sections Import Sections Import Section Sect	ections uplicate Sections	0	
Beam.2D file	erials and/or Sections from an existing EngiLab (.bea). Select the entities you would like to import ether duplicates should be ignored or not.	Browse Cancel	

The user can choose to import Materials and/or Sections and also whether duplicate Materials and/or Sections should be ignored.

3.2 Convert Model to Truss



This command converts a **Frame** model to a **Truss**. A Truss model has only axial tension (no shear, no bending moment).

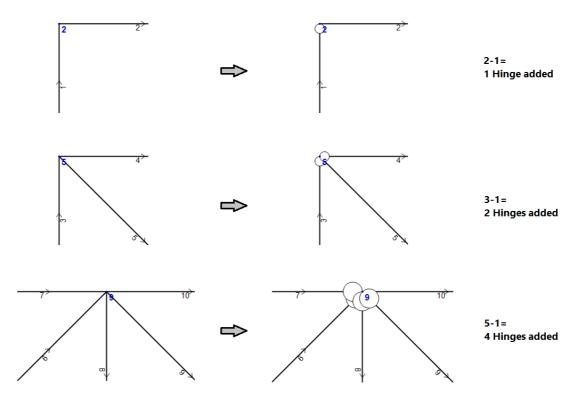
Important: Please note that if a Body (Acceleration) Load is present, then a Truss model can have also shear and bending, for example bending of a truss element due to its self weight. In this case, each element acts as a simply supported beam with uniform load on.

In order to convert a Frame model to a Truss, the program automatically makes the



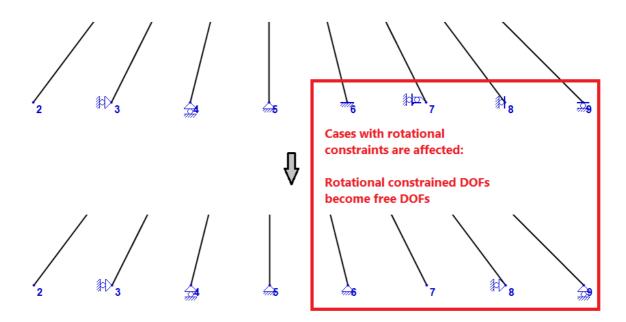
following changes to the model:

1. Every node where elements are connected is converted to a Hinge. When N elements are connected to a node, N-1 hinges are added to the N elements, as explained in detail in the following picture.



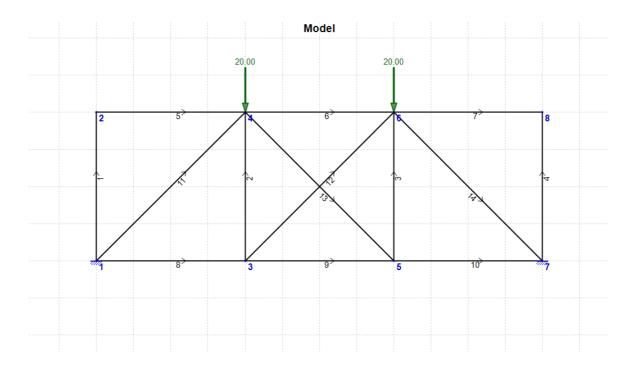
 Supports are converted to Truss-Model supports, by releasing any constraints on the rotational Degree of Freedom (RZ-Con). The other two DOFs of each node are not affected. The picture below explains this conversion for every support case in EngiLab Beam.2D.





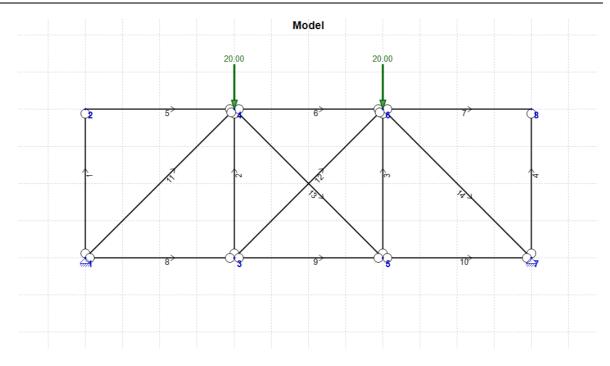
See the example below.

The picture below shows the initial Frame Model



The picture below shows the Model after it has been converted to a Truss







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Chapter



Analysis and Analysis results

4 Analysis and Analysis results

- <u>Analysis</u>
- N, V, M Diagrams
- Deformation
- Free Body Diagram (F)
- <u>Analysis results</u>
 - <u>Node Displacements</u>
 - Element End Forces
 - Support Reactions
 - Full Report (RTF)
 - Analysis Validation

4.1 Analysis

The analysis is performed using the **Finite Element Method (FEM)** for plane frames. During the analysis process, the program forms the model Global Stiffness Matrix [K] as well as the vector of the External Forces {F}. Then, the linear equations system {**F**}= [**K**]{**D**} is solved in order for the Displacement Vector {**D**} to be calculated:

 $\{\mathbf{D}\}=[\mathbf{K}]^{-1}\cdot\{\mathbf{F}\}$

If the model is statically well-defined then the Stiffness Matrix [K] is reversible (Matrix Determinant greater than zero) and no problems will occur during the analysis process.

Errors during the analysis process

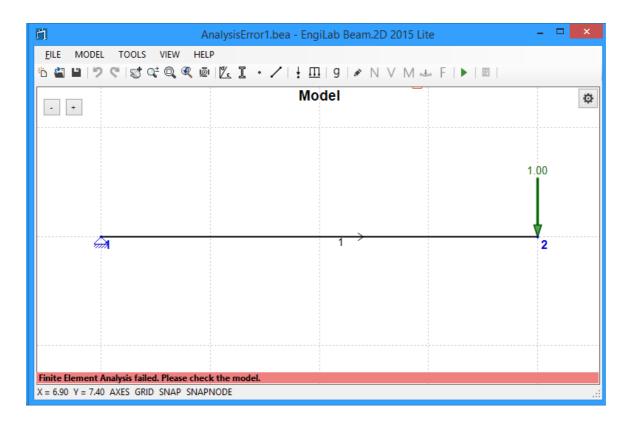
Computational errors may occur during the analysis process. In that case, the program warns the user with the following messages:

1. 'Finite Element Analysis failed. Please check the Model.'

This error occurs due to the model stiffness matrix being non-reversible. There is a zero-element appearing at the matrix diagonal, the determinant of the model stiffness matrix equals to zero, the matrix is non-reversible thus the linear equations system cannot be solved for displacements.



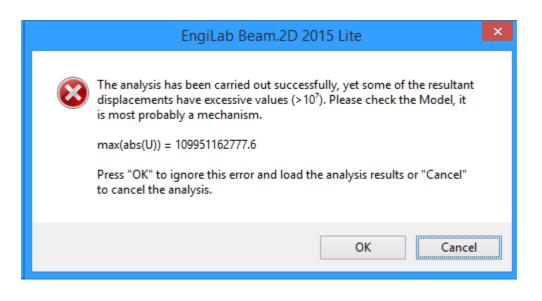
Example: Under-constrained model, such as a Cantilever beam where instead of a fixed support, there is a Pinned support as shown below. The model can freely rotate and Equilibrium cannot be reached, by no means. The model cannot be analyzed and the programs gives us an error message, as shown below.





2. 'The analysis has been carried out successfully, yet some of the resultant displacements have excessive values (> 10^7). Please check the Model, it is most probably a mechanism.'

This case does not have to do with a computational error occurring during the analysis process. The analysis has been carried out successfully yet some of the resultant displacements have excessive values (greater than 10^7) which means that probably the structure is a mechanism. In this case the model stiffness matrix is in fact non-reversible but during the analysis process instead of the right zero value, a very small non-zero value appears at the matrix diagonal due to computational approximation errors. As a result, the solver finds a solution (with no practical interest) to the linear equations system yet the resultant displacements have excessive values.





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- +	Model	
		1.00
	1 >	2
		N/
		3

This for example can happen in the Model shown below.

Possible reasons for such error messages:

- There are nodes that are not connected to the model via elements.
- The constraints are inadequate (under-constrained Model), for example the first model shown above.
- Some parts of the structure are not connected to each other and as a result the forces cannot be transferred from one part of the structure to another.
- The forces cannot be transferred from one part of a structure to another due to the type of the connecting elements (Hinges).
- Due to some other reason, the structure is a mechanism.



4.2 N, V, M Diagrams

<u>F</u>ILE MODEL TOOLS VIEW HELP [™] 🖆 💾 | 🎔 < ♥ | 😒

Q[±]

Q,

Q,

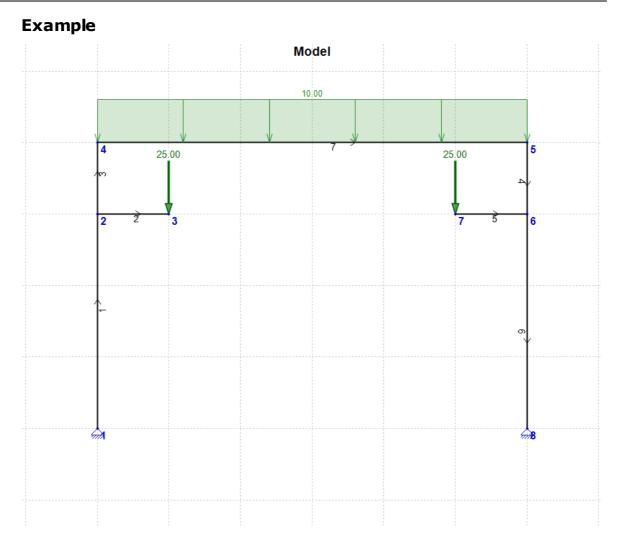
After setting up the model and analyzing it (Clicking the 'Analyze' button \blacktriangleright) you can click N, V or M to see the **Axial Force** Diagram, **Shear Force** Diagram or **Bending Moment** Diagram.

- N: Axial Force Diagram
- V : Shear Force Diagram
- M: Bending Moment Diagram

Note: Diagram values are given also on screen, if the mouse pointer moves over an element.

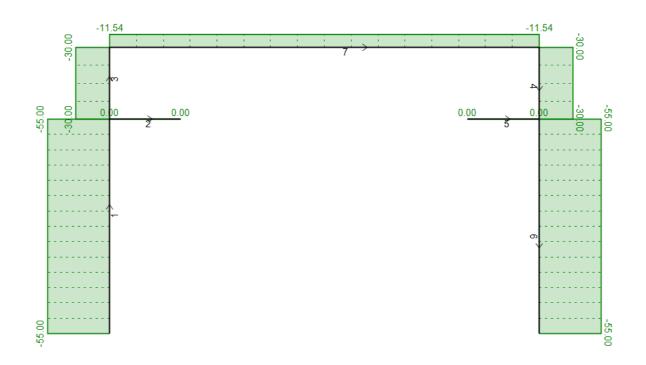
See also: <u>NVM Diagrams (Settings)</u>.







Axial Force Diagram [N]





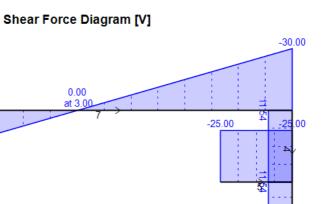
-11.54

-11.54

-11.54

2

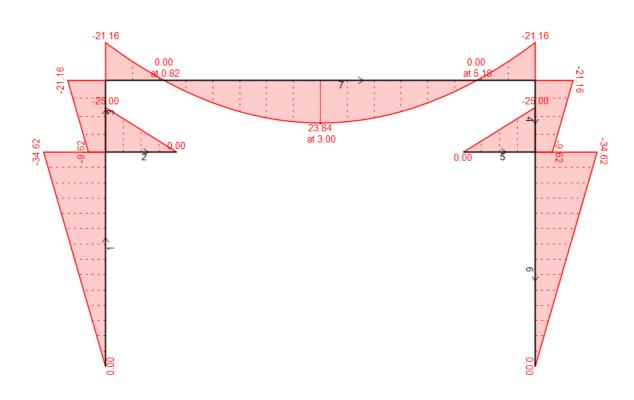
25.00



σ

1.54

Bending Moment Diagram [M]





4.3 Deformation

<u>F</u> ILE MODEL					
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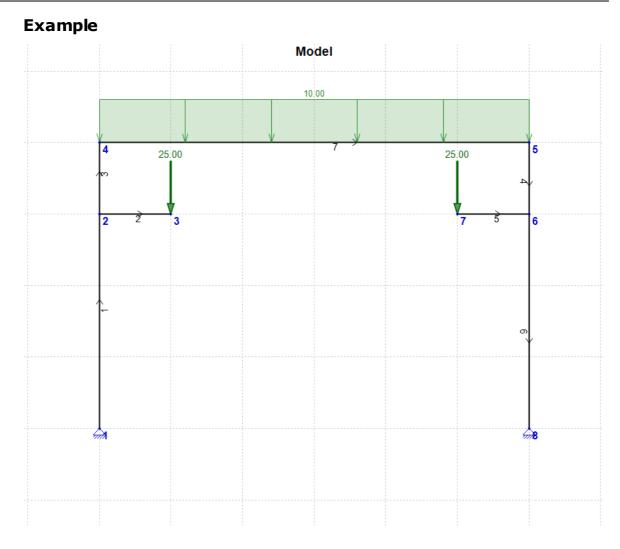
EngiLab Beam.2D uses the Finite Element Method (FEM) to analyze the Model. According to FEM, displacements are computed only for Nodes. Loads within elements (elemental loads) are distributed to the connecting Nodes and the analysis results give the Node Displacements vector (displacements at the positions of the Nodes). Calculating the intermediate displacements within elements and drawing accurately the deformed shape of the model is not an easy task, especially for cases of Elements with Hinges and linear varying loads on. EngiLab Beam.2D uses special computational techniques that give 100% accurate results without any approximations, even for the displacements along an element.

After setting up the model and analyzing it (Clicking the 'Analyze' button \triangleright) you can click \checkmark to see the deformed shape of the Model.

• 🏎: Deformation

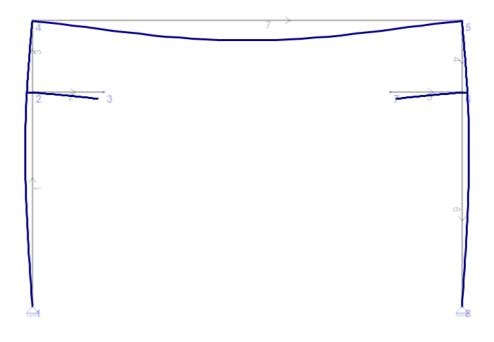
Note: Deformation values are given also on screen, if the mouse pointer moves over an element. The values that are given on screen are the x and y displacements of the corresponding point of each element. These are given in **Local Element Axes**, not Global axes.







Deformation (x15.57)



4.4 Free Body Diagram (F)

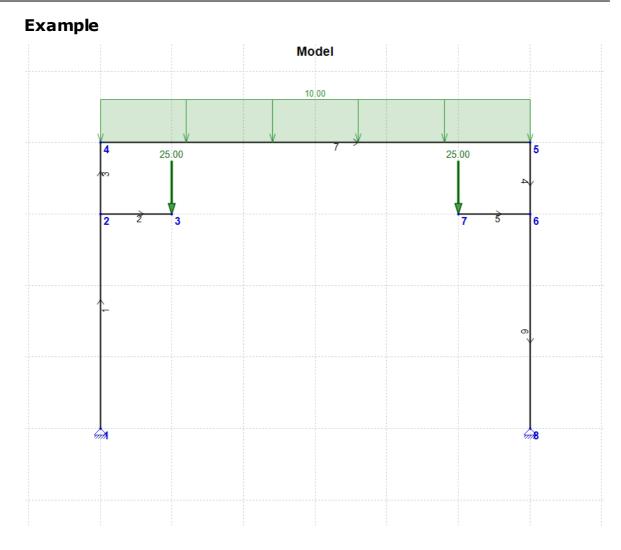
After setting up the model and analyzing it (Clicking the 'Analyze' button \triangleright) you can click F to see the Free Body Diagram of the Model.

• F : Free Body Diagram

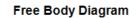
The Free Body Diagram shows the support reactions on screen and also the calculations of the equilibrium of the Model.

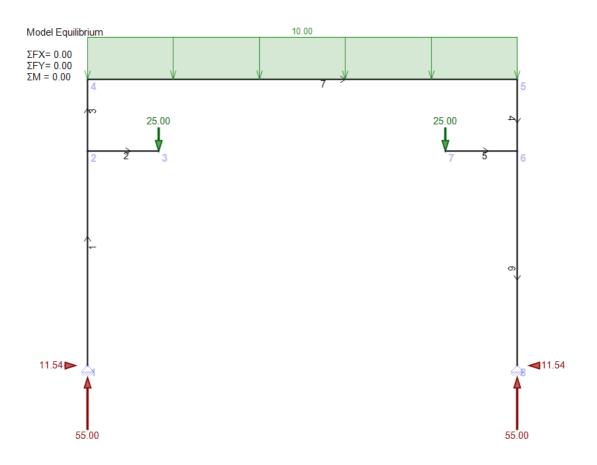
Note: If the mouse pointer moves over an element, the program shows the corresponding N, V and M values at the corresponding point along the element. The program shows also the Element End Forces and the calculations for the equilibrium of the specific Element.











4.5 Analysis results

_		TOOLS													
°C 🎦	2	C St 0)± 🔍 🤇	🤋 🧕 🕅	I	•	11	<u>m</u> g	۲	Ν	V	Мщ	F	F	

- <u>Node Displacements</u>
- Element End Forces
- Support Reactions
- Full Report (RTF)
- Analysis Validation



4.5.1 Node Displacements

Shows the Node Displacements of the Model. The sign convention is according to the Global Axes.

lode Displ	acements Element	End Forces S	upport Reactions	Full Report (RTF)	Analysis Validation	
Node ID	X-Displacement	Y-Displa	cement Z-R	otation	Cell co	
1	0.0000	0.0000	0.00	44	Constra	4 Displacer
2	-0.0049	-0.0002	-0.00	039		ined 4 🚔 Rotation
3	-0.0049	-0.0060	-0.00	068	Spring	2 🚔 Force
4	0.0001	-0.0002	-0.00	063	Positiv	× moment
5	-0.0001	-0.0002	0.00	63	Global a	ixes
6	0.0049	-0.0002	0.00	39		+
7	0.0049	-0.0060	0.00	68		→
8	0.0000	0.0000	-0.00)44	Res	at
						et
					0	
						ОК



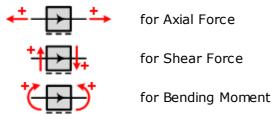
4.5.2 Element End Forces

Shows the End Forces of the Elements.

Node Displaceme	ents Elem	ent End Forces	Support Reacti	ions Full Report	t (RTF) Analys	is Validation	1
Element ID	Node ID	Axial Force	Shear Force	Bending Moment		Cell color:	Decimal places:
1 Start		-55.00	-11.54	0.00		Hinge	
1 End	2	-55.00	-11.54	-34.62		ringe	4 🜩 Rotation
2 Start	2	0.00	25.00	-25.00		Sign convention:	2 🔶 Force
2 End	3	0.00	25.00	0.00		Diagrams Local axes	2 🔺 Moment
3 Start	2	-30.00	-11.54	-9.62			
3 End	4	-30.00	-11.54	-21.16		<u>← →</u>	
4 Start	5	-30.00	11.54	-21.16			
4 End	6	-30.00	11.54	-9.62		│ `Ĩ<u></u><u>↓</u>↓	
5 Start	7	0.00	-25.00	0.00			
5 End	6	0.00	-25.00	-25.00			
6 Start	6	-55.00	11.54	-34.62		0	
6 End	8	-55.00	11.54	0.00			
7 Start	4	-11.54	30.00	-21.16			
7 End	5	-11.54	-30.00	-21.16			

There are two options for the sign convention:

 ${\bf 1}.$ Use the sign convention of the N, V, M diagrams. For example, the convention could be as shown below.



The above is only an example of these settings, as the sign convention for the diagrams can be changed by the user. See <u>NVM Diagrams (Settings)</u>.

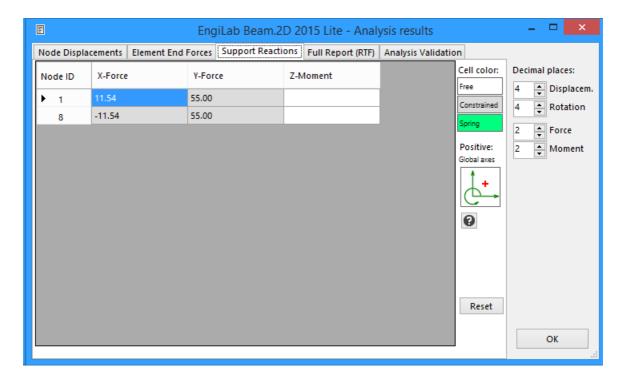
2. Use the sign convention of the Finite Element Analysis (Element local axes), as shown below. This sign convention cannot change.



4.5.3 Support Reactions

Shows the Support Reactions (Constraints or Springs) of the Model. The sign convention is in accordance with the Global Axes.

Special colors are used for every Degree Of Freedom (Free, Constrained or Spring), as shown below.





4.5.4 Full Report (RTF)

Shows all the Analysis results in Rich Text Format (RTF). You can easily copy the document and paste it anywhere, or you can click the "Save" button and save the document in RTF format.

In the latest version of the program, all the diagrams have also been included in the Full Report. The following pictures are added:

- Model
- Axial Force Diagram (N)
- Shear Force Diagram (V)
- Bending Moment Diagram (M)
- Deformation (D)
- Free Body Diagram (F)

			EngiLab Bear	ite - Analysis results 🚽 🗖
lode Displacements El	ement End Forces Su	pport Reactions	ull Report (RTF) Analy	
ngiLab Beam.2D 20				▲ V Displac. Decimal plac
lodel: "Frame 1.bea"				
eport created: Sunda	v. 10 May 2015, 16	20:22		4 015
	, , , , , , , , , , , , , , , , , , , ,			Reactions 4 Rot
ode Displacements				Color
				O B&W 2 🚔 For
Node ID X	(-Displacement Y	(-Displacement	Z-Rotation	
1	0.0000	0.0000	0.0044	
2	-0.0049	-0.0002	-0.0039	
3	-0.0049	-0.0060	-0.0068	
4	0.0001	-0.0002 -0.0002	-0.0063 0.0063	
6	0.0049	-0.0002	0.0039	
7	0.0049	-0.0060	0.0068	
8	0.0000	0.0000	-0.0044	
gn convention: Positive au splacements reported wit tations (in Radians) repo teckground color: Free [ement End Forces]	th 4 decimal places rted with 4 decimal place	ces		
Classet TD	Autol Course	Change Course	Deedler Newest	
Element ID 1 Start	Axial Force -55.00	-11.54	Bending Moment 0.00	
1 Start 1 End	-55.00	-11.54	-34.62	
2 Start	0.00	25.00	-25.00	
2 End	0.00	25.00	0.00	
3 Start	-30.00	-11.54	-9.62	
3 End	-30.00	-11.54	-21.16	
4 Start	-30.00	11.54	-21.16	
4 End	-30.00	11.54	-9.62	
5 Start	0.00	-25.00	0.00	
5 End	0.00	-25.00	-25.00	
6 Start 6 End	-55.00 -55.00	11.54 11.54	-34.62	
7 Start	-11.54	30.00	-21.16	
7 End	-11.54	-30.00	-21.16	
In convention: According Axial force: Positive whe Shear force: Positive whe Shear force: Positive whe Bending moment: Positive rces reported with 2 dec nding moments reported ckground color: Norma upport Reactions	n member is in tension en it rotates the member when the "bottom" fibe imal places with 2 decimal places	r clockwise	ber in compression)	
Node ID 1 8	X-Force 11.54 -11.54	Y-Force 55.00 55.00	Z-Moment	
ign convention: Positive a orces reported with 2 dec ending moments reported ackground color: Free [imal places with 2 decimal places			
			Model	
			10:00	



4.5.5 Analysis Validation

Shows the Validation of the Analysis results. If the Analysis results are correct, then the Model should be in equilibrium. The program calculates the Sum of Forces (X and Y direction) and the Sum of Bending Moments, for the entire Model (Global Equilibrium) and also for each Element separately. There are two available tabs:

1. Global (Model) Equilibrium

The program calculates the Sum of X-Forces, the Sum of Y-Forces and the Sum of Bending Moments (at Point X=0 and Y=0) for the whole Model, for the Nodal Loads, Elemental Loads, Acceleration (Body) Loads and Support Reactions. All three Sums (last row of the table, in bold) have to be zero if the Model is in Equilibrium.

	eactions Full Repo	- Analysis results ont (RTF) Analysis Validati Σ(MZ at 0,0) -350.00 -420.00 0.00 770.00 0.00	on Global axes	Decimal places: 4 Displacem. 4 Rotation 2 Force 2 Moment
				ОК



2. Individual Element Equilibrium

The program calculates the Sum of X-Forces, the Sum of Y-Forces and the Sum of Bending Moments at Start i and at End j of each Element, for the Elemental Forces, Body Forces, End Forces at i and End Forces at j. The Sum of X-Forces, the Sum of Y-Forces and also the Sums of Bending Moments at both Start i and End j (in bold) have to be zero if the Element is in Equilibrium.

Element ID	FX Elemental Force	FX Body Force	FX End Force i	FX End Force j	SUM ΣFX	FY Elemental Force	FY Body Force	FY End Force i	FY End Force j	SUM ΣFY	SUM SMZ at i	SUM ΣMZ at j	Decimal places: 4 Displac 4 Rotatio
▶ 1	0.00	0.00	11.54	-11.54	0.00	0.00	0.00	55.00	-55.00	0.00	0.00	0.00	2 🜩 Force
2	0.00	0.00	0.00	0.00	0.00	0.00	0.00	25.00	-25.00	0.00	0.00	0.00	2 🌲 Momen
3	0.00	0.00	11.54	-11.54	0.00	0.00	0.00	30.00	-30.00	0.00	0.00	0.00	
4	0.00	0.00	11.54	-11.54	0.00	0.00	0.00	-30.00	30.00	0.00	0.00	0.00	
5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	-25.00	25.00	0.00	0.00	0.00	
6	0.00	0.00	11.54	-11.54	0.00	0.00	0.00	-55.00	55.00	0.00	0.00	0.00	
7	0.00	0.00	11.54	-11.54	0.00	-60.00	0.00	30.00	30.00	0.00	0.00	0.00	



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Chapter



Settings

5 Settings

- <u>General</u>
- <u>Model</u>
- NVM Diagrams
- <u>Colors</u>
- Fonts
- <u>Decimals</u>
- <u>Results</u>

5.1 General (Settings)





Available settings:

- Global Axes: Shows/Hides the Global Axes
- Grid: Shows/Hides the Grid
- Grid size: Sets the size of the Grid
- **Snap Mode**: Enables/Disables the Snap Mode. If Snap Mode is enabled (Default=True), then you can only draw Nodes and Elements at increments defined by the Snap Size setting
- Snap Size: Sets the size of the Snap (Default=0.1)
- **Snap to Node**: Enables/Disables the Snap to Node. If Snap to Node is enabled (Default=True), then the user can "catch" Nodes on screen so that new elements can be connected to existing Nodes.
- Allow to check for updates online: Enables/Disables the automatic update feature. If disabled, the program will not check for updates online.

5.2 Model (Settings)

Fonts	Decimals	Res	ults		_	_
General	Model	NVM	Dia	grams	Colors	
Nodes group Elements group Image: Nodes Elements Image: Numbering Numbering Image: Constraints Image: Hinges Image: Coordinates Image: Orientation Image: Springs Image: Material Image: Spring values Section Image: Length Image: Section						
Loads group Image: Constraint of the second seco						
 Elem. L. values Nodal Loads representation Show the Resultant force Show the X, Y Components 						
Acceleration Acceleration Image: Constraint of the second secon						
0				Restor	e Default	s

Each setting Shows/Hides the corresponding object on screen.



5.3 NVM Diagrams (Settings)

Fonts Decimals Results General Model NVM Dia	
Show in NVM Diagrams Nodal values Min, Max values and p Zero values and position Intermediate values Intermediate lines Diagram Fill	ositions
Positive direction "Bottom" fiber "Top" fiber	i j
Axial Force positive Tension Compression	<u>+</u> - <u>}</u> -+→
Shear Force positive Clockwise Counter-Clockwise	⁺ <mark>┼<mark>┟</mark>╤┨┼</mark>
Bending Moment positive "Bot." fib. in tension "Top" fiber in tension	₹ <u>€</u> €}
0	Restore Defaults

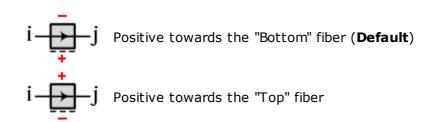
Available settings:

- **Nodal values**: Shows/Hides the Nodal values of the diagrams (Axial Force, Shear Force or Bending Moment).
- **Min, Max values and positions**: Shows/Hides the (local) Minimum and Maximum values of the diagrams and the corresponding positions along an Element.
- Zero values and positions: Shows/Hides the Zero values of the diagrams and the corresponding positions along an Element.
- Intermediate values: Shows/Hides the intermediate values of the diagrams.
- **Intermediate lines**: Shows/Hides the intermediate lines of the diagrams.
- Diagram Fill: Shows/Hides the diagrams fill.



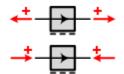
Positive direction (applies to all diagrams)

Determines the positive direction of the diagrams. Applies to all three diagrams (Axial Force, Shear Force or Bending Moment). There are two options:



Axial Force positive

Determines the sign convention for the **Axial Force**. There are two options:



Axial Force is Positive when member is in Tension (Default)

Axial Force is Positive when member is in Compression

Shear Force positive

Determines the sign convention for the **Shear Force**. There are two options:



Shear Force is Positive when member is rotated clockwise (**Default**)

Shear Force is Positive when member is rotated counter-clockwise

Bending Moment positive

Determines the sign convention for the **Bending Moment**. There are two options:



Bending Moment is Positive when the "Bottom" fiber is in tension $(\mbox{Default})$

Bending Moment is Positive when the "Top" fiber is in tension



5.4 Colors (Settings)

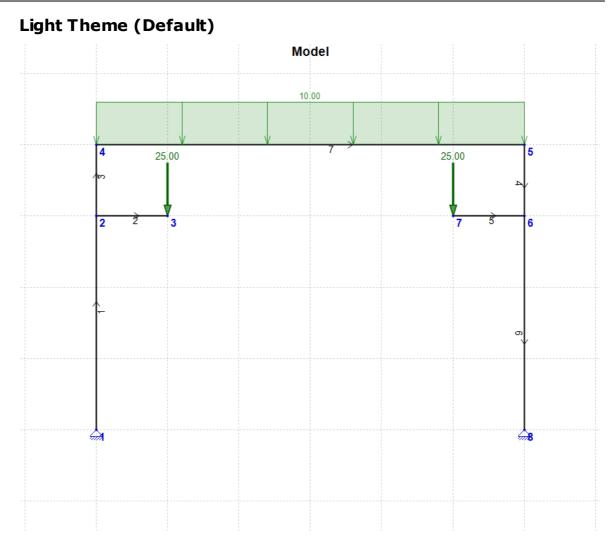
Fonts	Decimals Re	sults				
General Model NVM Diagrams Colors						
General Model						
Background Nodes						
Axes Elements						
Grid Nodal Loads						
		Elem. Loads				
	Acceleration					
Graphical User Interface						
A 💻	Active Node					
A	Active Element					
s 📃 s	Snap Rectangle					
Z	Zoom Window					
	Cursor values in NVMDF Drawings					
NVM Diagrams						
A	Axial Force Transparency % 80 🌲					
S	Shear Force Transparency % 80 🗣					
Bend. Mom. Transparency % 80						
D (Deformation) Drawing						
Deformed Model						
F (Free Body Diagram) Drawing						
Support Reactions						
E	lement End Fo	rces				
E	quilibrium Calo	culations				
0	Light Th	eme Dark Theme				

Each setting controls the color of the corresponding object on screen.

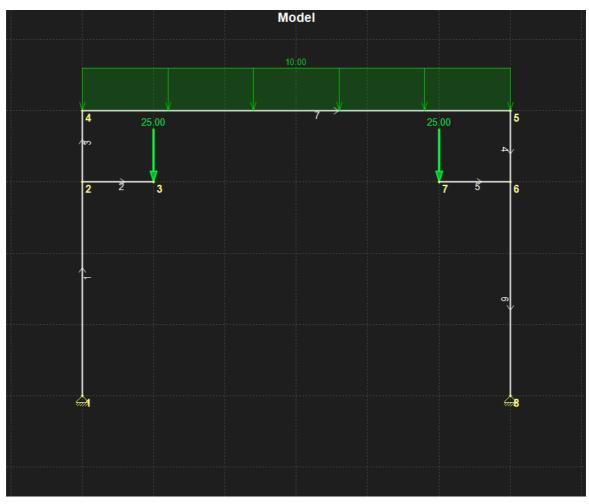
You can also set the transparency of the filled objects (N, V, M Diagrams fill).

There are two predefined color themes available, the **Light Theme (Default)** and the **Dark Theme**. The two themes are shown below.









Dark Theme



5.5 Fonts (Settings)

General	Model NVM Diagram	ns Colors
Fonts	Decimals Results	
Genera	al	
	Picture Tit	le 14 🌲
	Axe	es 13 🌲
Model		
	Node	es 12 🌲
	Elemen	ts 11 🌲
	Nodal Load	is 11 🌲
	Elemental Load	
	Acceleratio	
	Acceleratio	
)iagrams, D and F Drawi	
	NVM Nodal Value	es 11 🌲
	NVM Intermediate Value	es 10 🌲
	NVMDF Cursor Position	ns 11 🌲
	NVMDF Cursor Value	es 12 🌲
	F Support Reaction	ns 11 🌲
	F Element End Force	
F	Equilibrium Calculation	
0	Resto	re Defaults

Each setting sets the Font size for the corresponding object on screen.



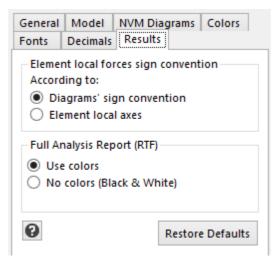
5.6 Decimals (Settings)

General	Model	NVM Diagrams	Colors
Fonts	Decimals	Results	
		Coordinate	2
		Length	2
		Displacement	4 🛓
		Rotation	4 🛓
		Force	2
		Moment	2
0		Restore D)efaults

These settings influence only the appearance of the Model input and the Analysis output.

Note: All available digits are taken into consideration for the analysis, at all times. There are no approximations

5.7 Results (Settings)



Available settings:

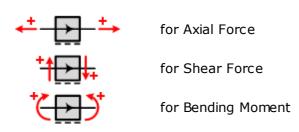
Element local forces sign convention

Determines how the sign of the element local forces will be reported in the Analysis Results. There are two options:

1. Use the sign convention of the N, V, M diagrams. For example, the convention

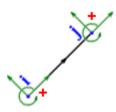


could be as shown below.



The above is only an example of these settings, as the sign convention for the diagrams can be changed by the user. See <u>NVM Diagrams (Settings)</u>.

2. Use the sign convention of the Finite Element Analysis (Element local axes), as shown below. This sign convention cannot change.



Full Analysis Report (RTF) - Use Color or Black & White

Applies only for the Full Analysis Report in RTF. You can choose to use color to highlight special items (Constraints, Springs, Hinges) or only Black and White without highlighting special items.





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Chapter



Useful information

6 Useful information

- Import DXF file
- <u>Ready-to-analyze Examples</u>
- Tips on Modeling hinges
- <u>Tips on Modeling symmetric structures</u>

6.1 Import DXF file

An alternative way to build a Model quickly, is by importing a DXF file. You can draw your Model in a CAD program using simple LINES (and optionally POINTS) and you can import your drawing into EngiLab Beam.2D as a structural Model.

In order to open the "Import DXF file" form, **you first have to define at least one Material and one Section**. All the LINES of the DXF file will be converted to Model Elements assigned Material 1 and Section 1.

₽x _F Import I	DXF file ×				
 Interpret LINE as Model Element (Nodes automatically added at both ends) Interpret POINT as Model Node Only add LINE or POINT with Z=0 Check if Node already exists and use existing node (for connection) Delete existing Nodes and Elements of the model Apply "Zoom All" function at the end, to fit the model to the screen 					
All new Nodes will be free (no constraints, no springs) with no loads on. All new Elements will be assigned the main Material (Material 1). All new Elements will be assigned the main Section (Section 1). All new Elements will have no hinges and no loads on.					
Supported DXF file versions:	0				
AutoCAD 2010 DXF AutoCAD 2007/LT2007 DXF AutoCAD 2004/LT2004 DXF AutoCAD 2000/LT2000 DXF AutoCAD R12/LT2 DXF	Browse				
AUTOCAD R12/L12 DXP					

Notes:

- All new Nodes will be free (no constraints, no springs) with no loads on.
- All new Elements will be assigned the main Material (Material 1).
- All new Elements will be assigned the main Section (Section 1).
- All new Elements will have no hinges and no loads on.



Supported DXF file versions:

- AutoCAD 2010 DXF
- AutoCAD 2007/LT2007 DXF
- AutoCAD 2004/LT2004 DXF
- AutoCAD 2000/LT2000 DXF
- AutoCAD R12/LT2 DXF

Most probably the DXF Import feature will work without any problems also with newer versions of AUTOCAD DXF files, yet we have not confirmed it yet.

6.2 Ready to-analyze Examples

۲. N	lodel1.bea - EngiLab Beam.2D 2015 Lite	- 🗆 🗙						
FILE MODEL TOOLS VIEW	HELP							
ቴ 當 💾 🤊 ୯ 😴 Q‡ Q, ଡ	View Help F1 🔰 🖍 N V M 🞿	F 🕨 🗉						
	Open Examples Folder							
	EngiLab on the web	\$						
	About EngiLab Beam.2D							
Y								
T T								
X								
^								
X = 2.80 Y = 2.30 AXES GRID SNAP								

In the folder **\Examples** (which is located where the program was installed), you can find ready-to-analyze EngiLab Beam.2D input files (*.bea) that you can open and analyze within EngiLab Beam.2D.

• There are 18 example files in total

How to open and Analyze an example:

- From the FILE menu, select Open.
- Open the Examples folder (Usually C:\Program Files (x86)\EngiLab\EngiLab Beam.2D 2015 Lite\Examples for the Lite Edition and C:\Program Files (x86) \EngiLab\EngiLab Beam.2D 2015 Pro\Examples for the Pro Edition.
- Select a .bea file to open.
- After the file is opened, click the 🕨 (Analyze) button to analyze the model.



75



After the analysis is finished:

Click $N,\ V$ or M to see the Axial Force Diagram, Shear Force Diagram or Bending Moment Diagram.

- N: Axial Force Diagram
- V : Shear Force Diagram
- M: Bending Moment Diagram

Click is to see the deformed shape of the Model.

• 📥: Deformation

Click F to see the Free Body Diagram of the Model.

• F : Free Body Diagram

Click I to see the analytical results. The results include the following tabs:

- Node Displacements
- Element End Forces
- Support Reactions
- Full Report (RTF)
- Analysis Validation

6.3 Tips on Modeling hinges

The hinges (rotational releases) of a model is an issue which requires our attention. There are two kinds of hinges: **External hinges (Pinned constraints)** and **Internal hinges**.

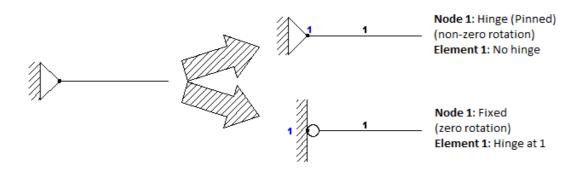
External hinges (Pinned constraints)

1. An external **hinge to which only ONE element is connected** can be given in a model using two possible ways (the result of the analysis should be the same):

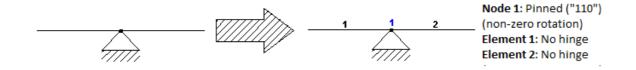
- A. As a pinned Node (Node 1 "110") connecting an Element with no hinge at end 1
- **B**. As a fixed Node (Node 1 "111") connecting an Element with a hinge at end 1.

Note that only by using the first option (A) you can get the rotation of the Element at end 1 in the analysis results.





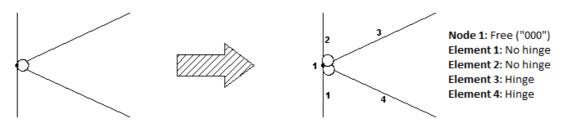
2. An **external hinge to which more than one elements are connected** must be given in the Model as follows:



Internal hinges

An **internal hinge** must be given always as a hinge of one or more elements as follows:

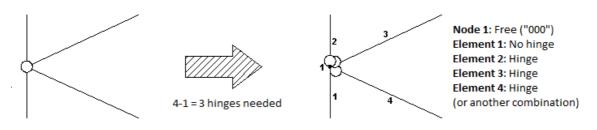
1. 'Partial' internal hinge (Applies to some of the connecting elements)





2. 'Full' internal hinge (Applies to all connecting elements)

A 'full' internal hinge which applies to all connecting elements means that for each of the connecting elements the bending moment value at the specific element end is zero. To model that case, you need as many as ('connecting elements' - 1) hinges for the connecting elements. Only one of the connecting elements should have no hinge at the specific end, no matter which of them.

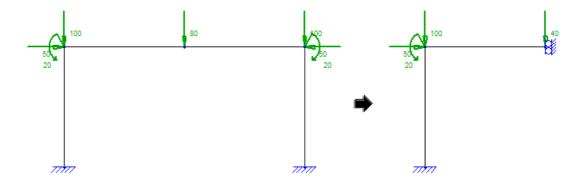


Note: In the case of a 'Full' internal hinge, each connecting element has its own rotation at the hinge end. The program only calculates the rotation of the element with no hinge. In the example below, the rotation of element 1 at end 1 will be calculated, while the rotations of the elements 2, 3 and 4 at end 1 will not be given in the analysis results. By using different combinations of releases, one can get the rotations of any connecting element separately.

6.4 Tips on Modeling symmetric structures

If a structure is symmetric (Symmetric structure with symmetric / anti-symmetric loads), then the size of the finite element model can be reduced, which, in turn, reduces the time and cost of the analysis. For each plane of symmetry in the model, the model size can be reduced by a factor of approximately two. See the examples below.

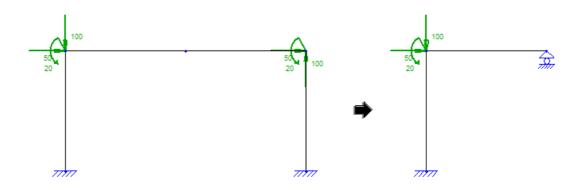
Case 1: Symmetric structure (axis) + Symmetric loads (axis)



For the above example, half the structure can be analyzed using a "101" constraint for the node on the symmetric axis. Note that if there is a load applied on that node, **half the load has to be applied on the constrained node of the second model**.

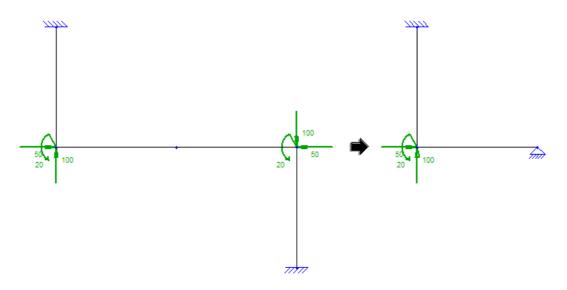


Case 2: Symmetric structure (axis) + Anti-Symmetric loads (axis)



For the above example, half the structure can be analyzed using an "010" (x-Roller) constraint for the node on the symmetric axis.

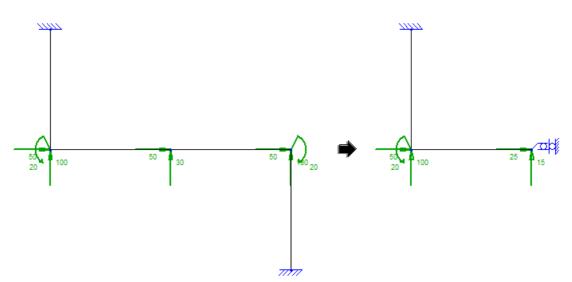
Case 3: Symmetric structure (point) + Symmetric loads (point)



For the above example, half the structure can be analyzed using a "110" (Pinned) constraint for the node on the symmetric point.



Case 4: Symmetric structure (point) + Anti-Symmetric loads (point)



For the above example, half the structure can be analyzed using a "001" constraint for the node on the symmetric point. Note that if there is a load applied on that node, **half the load has to be applied on the constrained node of the second model**.



Chapter



Example Problems

7 Example Problems

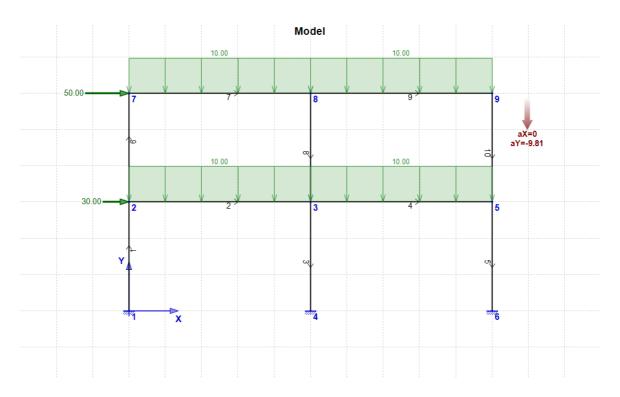
• Example Problem 1

7.1 Example Problem 1

- Overview Example 1
- Step 1. Preparation of the input data
- <u>Step 2. Define Materials</u>
- <u>Step 3. Define Sections</u>
- Step 4. Draw the Model on screen
- Step 5. Edit Nodes
- Step 6. Edit Elements
- Step 7. Define Nodal Loads
- Step 8. Define Elemental Loads
- Step 9. Define Body (Acceleration) Loads
- Step 10. Run the Analysis
- Step 11. View N, V, M Diagrams, Model Deformation and Free Body Diagram
- Step 12. View the analytical results

7.1.1 Overview - Example 1

The first example is a **two-story**, **two-bay concrete frame** which is shown in the figure below.





The properties of the Model are the following:

Materials

The material of the Model is **Concrete** with the following properties:

- Elastic Modulus **E = 29 GPa**
- Density **d = 2500 kg/m³**

Sections

There are different sections for the **Columns** and **Beams**:

Columns: Square section, 50 cm * 50 cm

Beams: Rectangular section, **50 cm * 25 cm** (bending in the major axis - 50 cm is the height of the beam (along the Global Y axis) and 25 cm is its width (perpendicular to the screen))

Member geometry (Nodes and Elements)

As shown in the Figure above, where **the Grid is 1 m**. So the **height of each floor is 3 m** and each bay has a **span of 5 m**. The total height of the structure is 6 m, while the total width is 10 m.

Elements' orientation is also shown in the figure (It is not very important to define the elements' orientation as shown above).

Nodal Loads

There is a **FX=30 kN** Load acting on **Node 2** and also a **FX=50 kN** Load acting on **Node 7**, as shown in the figure above.

Elemental Loads

All the beams have a uniform elemental load of **10 kN/m** at the direction **-Y (Global axes)**, as shown in the figure.

Body (Acceleration) Loads

The **self-weight** of all the structural elements (columns and beams) has to be taken into consideration in the analysis.

The earth gravitational acceleration is given equal to $g=9.81 \text{ m/s}^2$ (a typical value).



7.1.2 Step 1. Preparation of the input data

The first step is to define the **System of Units**. This is a very important step for the model. This step has nothing to do with the program itself. Instead, the user has to define the preferred system of units and then all the program data have to be consistent with this system. Then the results will also comply to that system. For details, see <u>System of Units</u>.

We choose a consistent system based on Force. We choose to use:

- m for Length (L)
- **kN** for **Force** (F)
- s for Time (T)

The derived units are then the following:

- The Acceleration unit is given by: L/T^2 (1 Length unit) / (1 time unit)^2. In our example: m/s^2
- The Mass unit is given by: F·T²/L (1 force unit) / (1 acceleration unit). In our example: kN/(m/s²) = Mg = t (IMPORTANT: t has to be used instead of kg!)
- The **Density unit** is given by: F•T²/L⁴ (1 mass unit) / (1 length unit)³. In our example: t/m³ (IMPORTANT: t/m³ has to be used instead of kg/m³!)
- The **Stress unit** is given by: F/L^2 (1 force unit) / (1 length unit)². In our example: kN/m^2

Note: The mass unit (in our example 1 t) is the mass that accelerates by the acceleration unit rate (in our example 1 m/s^2) when the unit force (in our example 1 kN) is exerted on it.

The Material properties should be given as:

- E = 29 GPa = **29000000** kN/m²
- **d** = 2500 kg/m3 = **2.5** t/m³

The Section properties should be given as:

- Columns: Square section, 50 cm * 50 cm = 0.50 m * 0.50 m
- Beams: Rectangular section, 50 cm * 25 cm = 0.50 m * 0.25 m

The gravity acceleration should be given as $g = 9.81 \text{ m/s}^2$

7.1.3 Step 2. Define Materials

After the preparation of the input data, it is time to start working with the program. Click \mathbb{Z}_{ϵ} to define **Material**(s). For details, see <u>Materials</u>.



You need to define the Material **Name** (optional), **Material Density d** (optional in general, but obligatory if you need to define self-weight loads, as in our case) and Material **Elastic Modulus E** for each Material, as shown below.

Žε.			Mate	erials		-		×
Mat. ID	Name		Density d		El. Modulus E			
Add new I	Material	Actions for se	elected Sele	ction to	ols		_	
New Mate	erial prop	erties				0		
Name Co	ncrete	d = 2.5		Add	new Material			
		E = 290	00000				ОК	



Then **Click the "Add new Material" button**. The new Material will be added to the table that shows the Model Materials.

Σ <u>ε</u>		-	□ ×		
Mat. ID	Name	Density d	El. Modulus E		
▶ 1	Concrete	2.5	2900000		
Add new M	aterial Actions for se	lected Selection t	ools		
New Mater	ial properties			0	
Name	d =	Add	new Material		
	E =				ОК

Then Click OK to exit the Materials form.

7.1.4 Step 3. Define Sections

After the definition of the Materials, you need to define the Sections of the Model. Click \mathbb{I} to define **Sections**(s). For details, see <u>Sections</u>.

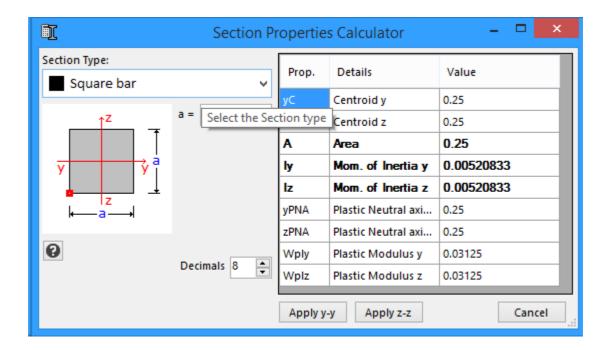


You need to define the Section **Name** (optional), Section **Area A** and Section **Moment of Inertia I** for each Section, as shown below. First, type the name of the Section. Let's start with the Columns.

I		Sections		-	□ ×
Sec. ID	Name	Area A	Mom. of Inertia I		
Add new S	Section Actions for se	lected Selection to	ola		
	on properties			0	
Name Co	lumns A =		Calculator		
	=	Add	new Section		ОК



You can calculate the Section properties (A and I) yourself, but the easiest way is to use the "Section Properties Calculator" that is built in EngiLab Beam.2D. **Click the** "**Calculator" button**. The Section Properties Calculator form appears. Select the Square Bar Section and type "0.5" for the a dimension, as shown below. Press ENTER after you have finished, in order to refresh the table with the Section properties.





Because of the fact that the Section is symmetric, it does not matter if you click "Apply y-y" or "Apply z-z" at this point, as the y and z properties of the section are the same. Let's **Click "Apply y-y"**. Then A and Iy are transferred to the main Section form, as shown below.

I		Sections		-	×
Sec. ID	Name	Area A	Mom. of Inertia I		
Add new S	Section Actions for	selected Selection t	ools		
New Sect	on properties	F	Calculator	0	
Name Co	olumns A = 0	0.25			
	I = 0	0.00520833 Ad	d new Section		ОК



Now **Click the "Add new Section" button**. The new Section will be added to the table that shows the Model Sections.

I			Sec	ctions		-		x
Sec. ID	Name		Area A		Mom. of Inertia	I		
▶ 1	Colum	ns	0.25		0.00520833			
Add ne	w Section	Actions for se	lected Sel	lection to	ols	_		
New S	ection prop	perties		F	Calculator	•		
Name		A =		91	calculator			
		=		Add	new Section		01	
-								

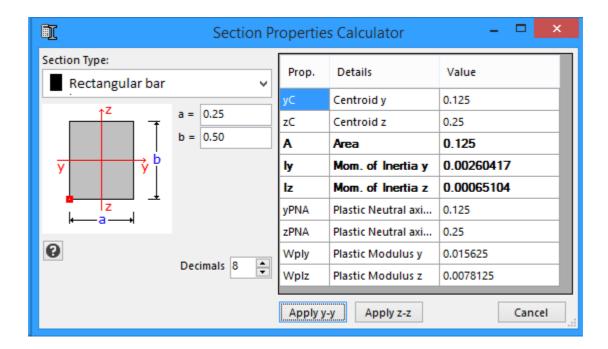


You now have to continue and add the Section of the Beams. **Type "Beams"** at the Section name, as shown below.

I		Sections		- 0	×
Sec. ID	Name	Area A	Mom. of Inertia I		
▶ 1	Columns	0.25	0.00520833		
Add new 9	Section Actions for sel	ected Selection to	ols	_	
New Secti	on properties		Calculator	0	
Name Be			carculator		
	Name of the ne	w Section Add	new Section		ок



You can use the Section Properties Calculator again. **Click the "Calculator" button**. The Section Properties Calculator form appears. Select the **Rectangular Bar Section** and type "0.25" for the a dimension and "0.50" for the b dimension, as shown below. Press ENTER after you have finished, in order to refresh the table with the Section properties.





The bending of the beam is about its strong axis (y-y), so this time you have to **Click "Apply y-y"** button. Then A and Iy properties are transferred to the main Section form, as shown below.

I		Sections		-	
Sec. ID	Name	Area A	Mom. of Inertia I		
▶ 1	Columns	0.25	0.00520833		
Add new 9	Section Actions for se	lected Selection to	ols		
New Sect	on properties		Calculator	0	
Name Be	ams A = 0.12	25	carculator		
	I = 0.00	0260417 Add	new Section		ок



Now **Click the "Add new Section" button**. The new Section will be added to the table that shows the Model Sections.

I		Sections		- 🗆 🗙
Sec. ID	Name	Area A	Mom. of Inertia I	
1	Columns	0.25	0.00520833	
▶ 2	Beams	0.125	0.00260417	
Add new S		ected Selection to	ols	
	on properties		Calculator	0
Name	A =		new Section	ОК

Then Click OK to exit the Sections form.

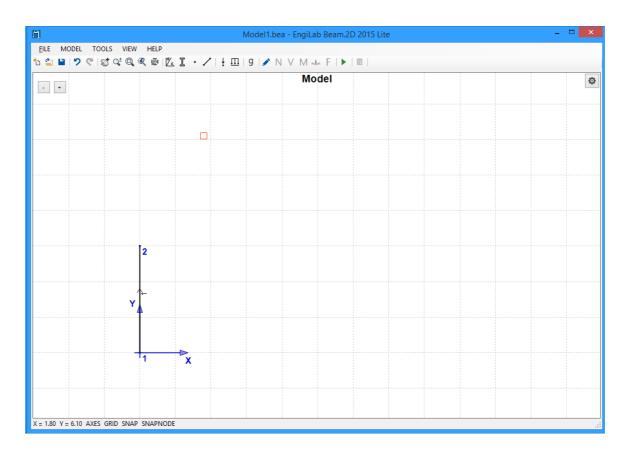
7.1.5 Step 4. Draw the Model on screen

After you have defined the Material and the Sections, you can **start drawing your Model on screen**, as follows:

- First make sure that **SNAP** and **SNAPNODE** are both activated (see bottom right of Picture below).
- If **SNAP** is activated (Default=True), then you can only draw Nodes and Elements at increments defined by the Snap Size setting (Default=0.1). This is fine for our example.
- If **SNAPNODE** is activated (Default=True), then you can "catch" Nodes so that new elements can be connected to existing Nodes. This is essential for building our Model.
- Left-click on screen, hold down the left button and then release it at another location to define a new Element and two nodes at ends i, j.
- Note: All Elements that are defined on screen are assigned Material 1 and Section 1. We will correct this later on.



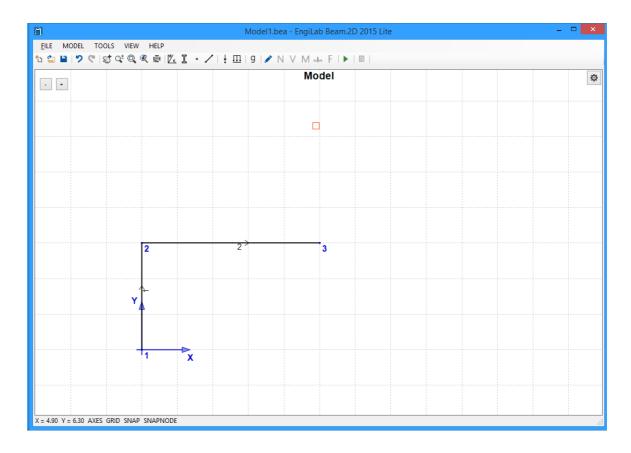
You can start at any point. Let's start from point 0,0 (Origin of axes). Draw the first column as shown below. Release the left button after you have done a distance of 3 in the Y Direction.





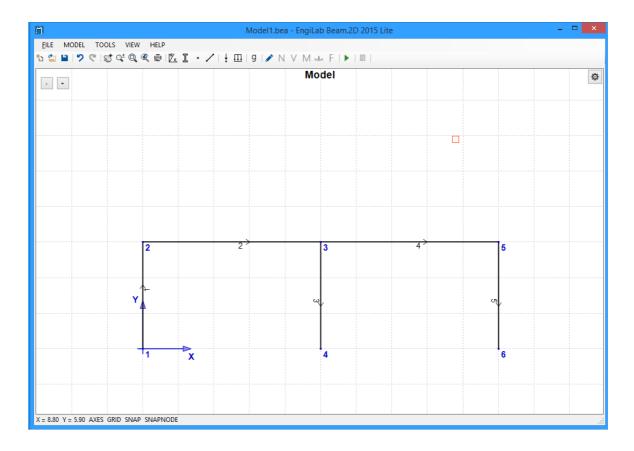


Continue with the Beam. Hold down the left button starting at Node 2 and release it at distance 5 in the X Direction. The picture should be as follows.



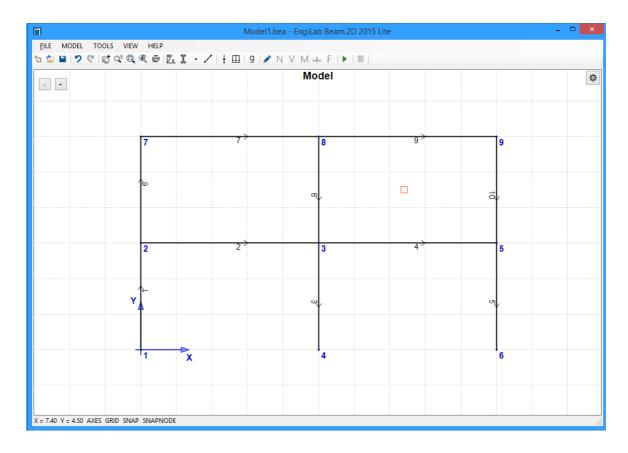


Using the same technique, continue with the other Columns and Beams of the first story. The picture should be as follows.









Continue with the Columns and Beams of the second story. Finally the picture should look as follows.

That's it! Now you have built the basic Model. But you need to apply some corrections. For example, now all Elements are assigned Section 1 (Columns), but the horizontal members have to be corrected as they must be assigned Section 2 (Beams). Also, loads (Nodal and Elemental) and also supports have to be added to the Model.

7.1.6 Step 5. Edit Nodes



After the preparation of the Model on screen, you will need to do some corrections on the Nodes that have been generated.

For example you may want to:

- Move Nodes to their exact positions, if needed.
- Define or change Nodal Constraints (Supports).
- Define Springs.



٠				No	odes			-	□ ×
Node ID	x	Y	DX-Con	DY-Con	RZ-Con	DX-Stiff	DY-Stiff	RZ-Stiff	
▶ 1	0	0				0	0	0	
2	0	3				0	0	0	
3	5	3				0	0	0	
4	5	0				0	0	0	
5	10	3				0	0	0	
6	10	0				0	0	0	
7	0	6				0	0	0	
8	5	6				0	0	0	
9	10	6				0	0	0	
Add new l	Node Arti	ons for selec	ted Sele	ction tool	c.				
					-			0	
X= Y=		Add new	Node						ОК

Click * to edit **Nodes**. For details, see <u>Nodes</u>.



In our example, there is no need to move Nodes, as they are already in their correct positions. What we need to do is to add Constraints (Supports) to the Model. **Set the Constraints for Nodes 1, 4 and 6** as follows.

•				No	odes			-	
Node ID	x	Y	DX-Con	DY-Con	RZ-Con	DX-Stiff	DY-Stiff	RZ-Stiff	
1	0	0	✓	<	✓	0	0	0	
2	0	3				0	0	0	
3	5	3				0	0	0	
4	5	0	✓	✓	✓	0	0	0	
5	10	3				0	0	0	
▶ 6	10	0	✓	✓	✓	0	0	0	
7	0	6				0	0	0	-
8	5	6				0	0	0	_
9	10	6				0	0	0	
Add new	Node Actio	ons for selec	ted Sele	ction tool	5				
X=	de coordina	ates Add new	Node					•	
Y=									OK



		Model1.	bea - EngiLab Beam.2D 2015	Lite		- - ×
	OOLS VIEW HELP					
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	<u>6</u>					
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					6	
	⁴⁴ 1 x		4		6	
	ES GRID SNAP SNAPNODE					

Then **Click OK**. The Model should now look like this:

Fixed supports have been added to Nodes 1, 4, 6, as shown above.

7.1.7 Step 6. Edit Elements



Now let's correct the Elements. For example you may want to:

- Assign the right Material and Section to every Element, if needed. All Elements that are defined on screen are assigned Material 1 and Section 1.
- Define Hinges at Element ends. Each Element can have a hinge at either end (Start-i or End-j or both).



/		Eleme	ents			-	
Elem. ID	Material	Section	Node i	Node j	Hinge i	Hinge j	Length
▶ 1	1. Concrete	1. Columns	1	2			3.00
2	1. Concrete	1. Columns	2	3			5.00
3	1. Concrete	1. Columns	3	4			3.00
4	1. Concrete	1. Columns	3	5			5.00
5	1. Concrete	1. Columns	5	6			3.00
6	1. Concrete	1. Columns	2	7			3.00
7	1. Concrete	1. Columns	7	8			5.00
8	1. Concrete	1. Columns	8	3			3.00
9	1. Concrete	1. Columns	8	9			5.00
10	1. Concrete	1. Columns	9	5			3.00
Add new	Element Actions for	selected Selection tool	c .				
	nent properties	Selected Selection tool	3				2
1	1. Concrete ∨ 1. Columns ∨	Node i Node j	Add nev	v Element			OK

Click 🖊 to edit **Elements**. For details, see <u>Elements</u>.



In our example, we need to assign the right Section (Beams) to the horizontal members. There are no hinges. Edit **Elements 2, 4, 7 and 9** and assign them the second Section (Beams) as follows.

/ Elements – 🗆 🗙									
Elem. ID	Material	Section	Node i	Node j	Hinge i	Hinge j	Length		
1	1. Concrete	1. Columns	1	2			3.00		
▶ 2	1. Concrete	2. Beams	2	3			5.00		
3	1. Concrete	1. Columns	3	4			3.00		
4	1. Concrete	2. Beams	3	5			5.00		
5	1. Concrete	1. Columns	5	6			3.00		
6	1. Concrete	1. Columns	2	7			3.00		
7	1. Concrete	2. Beams	7	8			5.00		
8	1. Concrete	1. Columns	8	3		3.00 5.00			
9	1. Concrete	2. Beams	8 9						
10	1. Concrete	1. Columns	9	5			3.00		
Add new Element Actions for selected Selection tools									
New Elen	nent properties	lodei Nodej					9		
Material Section	Material 1. Concrete V								

Then **Click OK** to exit Elements.

7.1.8 Step 7. Define Nodal Loads

<u>F</u> ILE	MODEL	TOOLS	VIEW	HELP							
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Click $\frac{1}{2}$ to define **Nodal Loads**. For details, see <u>Nodal Loads</u>.



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The	Nodal	Loads	form	appears.
THC	nouur	Louus	101111	uppeurs.

÷		N	odal Loads		- (×
	Node ID	Force FX	Force FY	Moment MZ		
Add new l	Nodal Load	Actions for selec	ted Selection to	ols		
New Nod			FX=		0	
Node		Cartesian (FX, FY) Polar (F, θ)	FY=	Add New		
			MZ=			ОК



÷		N	odal Loads			×
	Node ID	Force FX	Force FY	Moment MZ		
Add new I	Nodal Load	Actions for selec	ted Selection to	ols		
New Nod			FX= 30		0	
Node 2		Cartesian (FX, FY) Polar (F, θ)		Add New		
			MZ=		OK	



÷		Ν	odal Loads			×
	Node ID	Force FX	Force FY	Moment MZ		
▶ 1	2	30				
Add new 1	Nodal Load	Actions for selec	ted Selection to	ools		
New Nod	al Load		FX= 50		0	
Node	ID O	Cartesian (FX, FY)		Add New		
7		Polar (F, θ)	MZ=		(ОК

Click "Add Nodal Load". Then add a Nodal Load FX=50 for Node 7.



ŧ

1 2

New Nodal Load

Node ID

	N	odal Loads		-	×
Node ID	Force FX	Force FY	Moment MZ		
2	30				
7	50				

Click "Add Nodal Load". Now the Nodal Loads should look like this.

Add new Nodal Load Actions for selected Selection tools

Cartesian (FX, FY)

O Polar (F, θ)

FX=

FY=

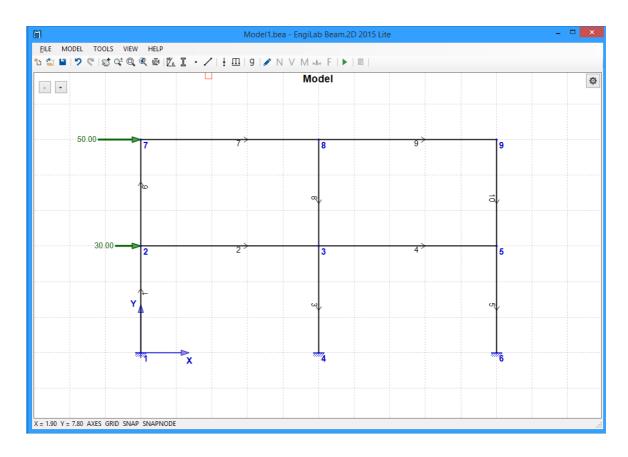
MZ=



0

OK

Add New



Click OK to exit. Now the Model should look like this.

7.1.9 Step 8. Define Elemental Loads

Click \coprod to define **Elemental Loads**. For details, see <u>Elemental Loads</u>.



The Elemental Loads form appears.

Ξ			Elemental Loa	ads	- 🗆 🗙
	Elem. ID	Force fXi	Force fXj	Force fYi	Force fYj
Add new f	Elemental Lo	ad Actions for s	elected Selectio	n tools	
	ental Load				0
Elem. ID	Uniform	fX=	Ac	ld Elem. Load	
	Linear varyin	ig fY=			ОК





Ξ			Elemental Loa	ids	_ c	×
	Elem. ID	Force fXi	Force fXj	Force fYi	Force fYj	
	Elemental Lo	ad Actions for s	elected Selectio	n tools		
New Elem Elem. ID	ental Load	EV -	1		0	
	- Uniform Linear varyin	fX= g fY= -10	Ad	d Elem. Load		ок

bbA	а	Uniform	Elemental	Load	fY = -	10	for	Element	: 2.
nuu	u	0	Licification	Loua				LICINCIN	

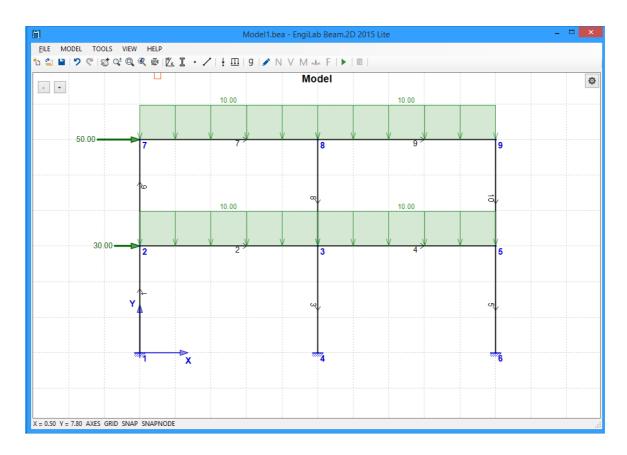


Click "Add Elemental Load". Then using the same technique, add Elemental Loads also for **Elements 4, 7, 9**. After you finish, you should see the following picture:

Ξ			Elemental Lo	ads		×			
	Elem. ID	Force fXi	Force fXj	Force fYi	Force fYj				
1	2			-10	-10				
2	4			-10	-10				
3	7			-10	-10				
▶ 4	9			-10	-10				
	Elemental Lo	ad Actions for s	selected Select	on tools	9				
Elem. ID	New Elemental Load								







Click OK to exit. Now the Model should look like this.



7.1.10 Step 9. Define Body (Acceleration) Loads

Click 9 to define **Body (Acceleration) Loads**. For details, see <u>Body (Acceleration)</u> <u>Loads</u>.

•		Body (Acceleration)	Loads		×		
Elem. ID	Material Density	Section Area	X-Body Load	Y-Body Load	Length			
▶ 1	2.5	0.25	0	0	3.00			
2	2.5	0.125	0	0	5.00			
3	2.5	0.25	0	0	3.00			
4	2.5	0.125	0	0	5.00			
5	2.5	0.25	0	0	3.00			
6	2.5	0.25	0	0	3.00			
7	2.5	0.125	0	0	5.00			
8	2.5	0.25	0	0	3.00			
9	2.5	0.125	0	0	5.00			
10	2.5	0.25	0	0	3.00			
Linear Ac aX =	Linear Acceleration Vector							
a¥ =		Delete	celeration vector	-	ОК			

We want to take into account the self-weight of Elements as an additional elemental load for each Element. So we have to provide the **Material Density** for the Material of each Element and also to define a Linear Acceleration Vector equal to the standard earth gravitational acceleration. We should put the earth gravitational acceleration with a minus (-) sign at the Y direction - this means gravity acting towards -Y global axis.



We are using kN for forces, m for length and s for time, so the Material Density has to be given in t/m³ (we have already done that) and we need to **input -9.81 at the aY component of the Linear Acceleration Vector**. After you type -9.81, make sure that you **press the ENTER key** in order for the changes to take effect. You should see the following picture.

•		Body	(Acceleration)	Loads	- 🗆 🗙
Elem. ID	Material Density	Section Area	X-Body Load	Y-Body Load	Length
▶ 1	2.5	0.25	0	-6.13125	3.00
2	2.5	0.125	0	-3.065625	5.00
3	2.5	0.25	0	-6.13125	3.00
4	2.5	0.125	0	-3.065625	5.00
5	2.5	0.25	0	-6.13125	3.00
6	2.5	0.25	0	-6.13125	3.00
7	2.5	0.125	0	-3.065625	5.00
8	2.5	0.25	0	-6.13125	3.00
9	2.5	0.125	0	-3.065625	5.00
10	2.5	0.25	0	-6.13125	3.00
Y-Acceler	ation set to -9.81				
Linear Act aX =	0				
aY =	-9.81	Delete			ОК

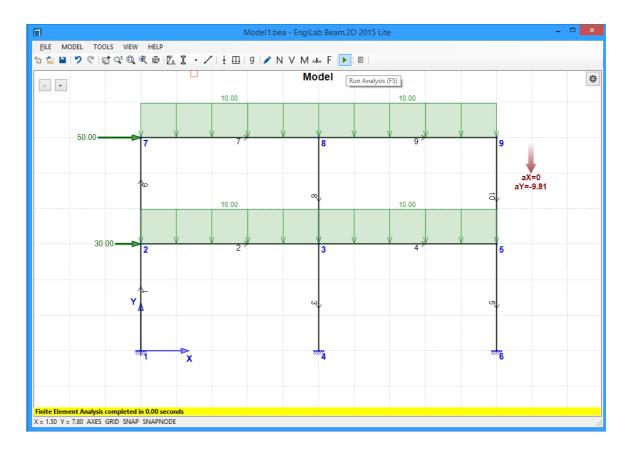
You see that the program has automatically calculated the self weight (in kN/m) for each Element. This is a read-only property. Click OK to exit the Body (Acceleration) Loads.



7.1.11 Step 10. Run the Analysis

Click (or press F5) to **run the Finite Element Analysis**. For details on the Analysis, see <u>Analysis</u>.

You should see the following picture.



The Finite Element Analysis has been completed successfully.

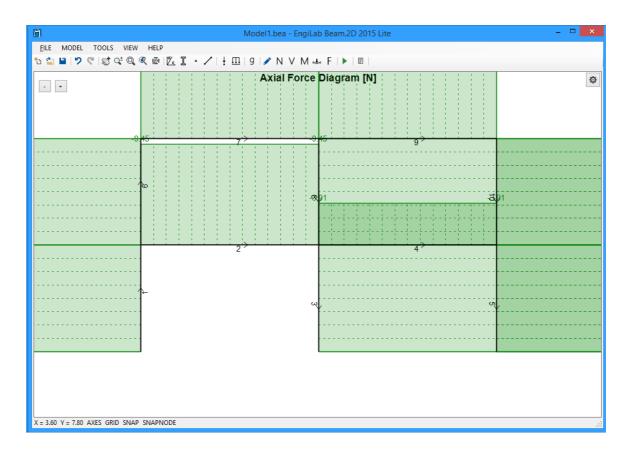


7.1.12 Step 11. View N, V, M Diagrams, Model Deformation and Free Body Diagram

<u>F</u> ILE MODEL	TOOLS V	VIEW HELP		Post-Processing Tools
ት 省 🗎 🤊 🛙	୯ 🕰 ପ‡	Q 🍳 🖻 🛛	I · / ! 9 #	N V M 🕹 F 🕨 🗉

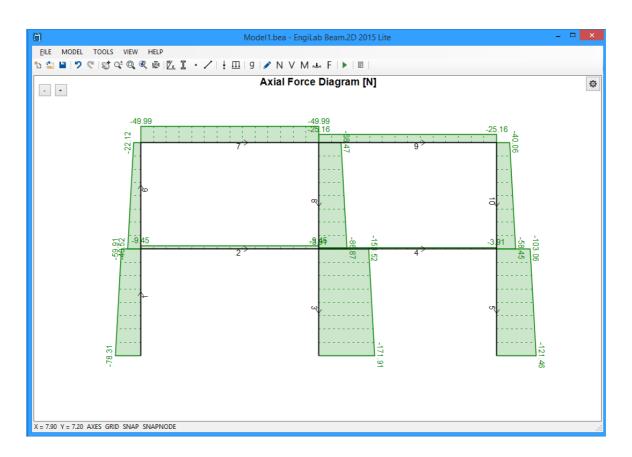
1. Axial Force Diagram

Click N, to see the Axial Force Diagram. You may take the picture below.



The Axial Force Diagram is out of Scale. Click the "Zoom All" button \overline{P} to automatically scale the Diagram.



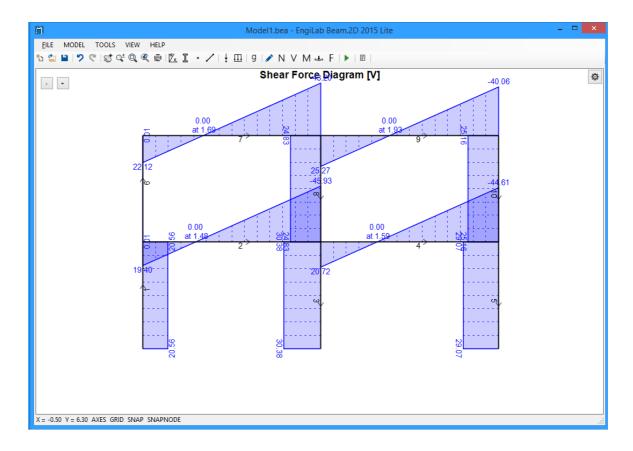


Now the result should look like this, which is much better.



2. Shear Force Diagram

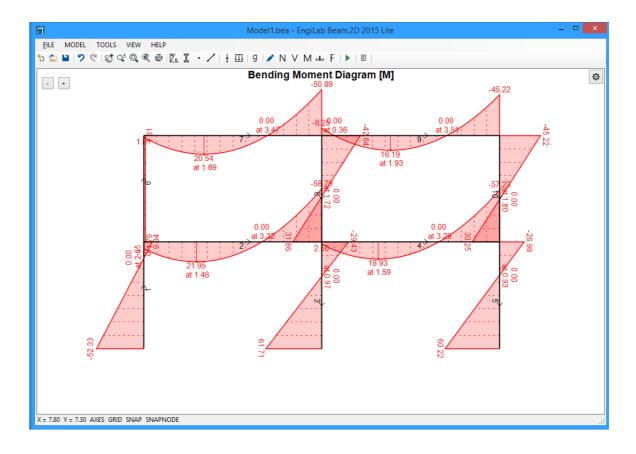
Click V, to see the Shear Force Diagram. If the Shear Force Diagram is out of Scale, **Click the "Zoom All" button** in to automatically scale the Diagram. You should take the following picture.





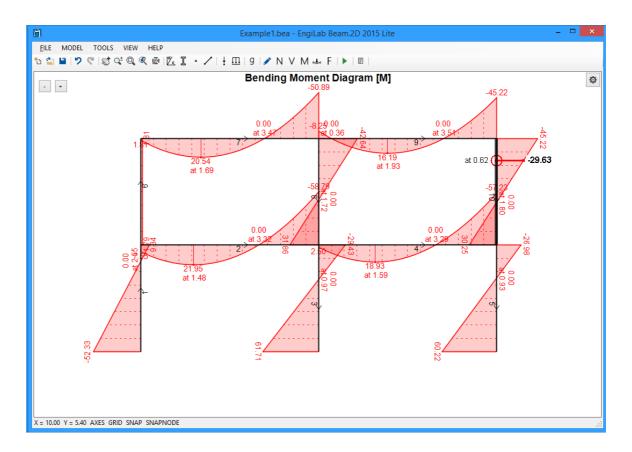
3. Bending Moment Diagram

Click M, to see the Bending Moment Diagram. If the Bending Moment Diagram is out of Scale, **Click the "Zoom All" button** in to automatically scale the Diagram. You should take the following picture.





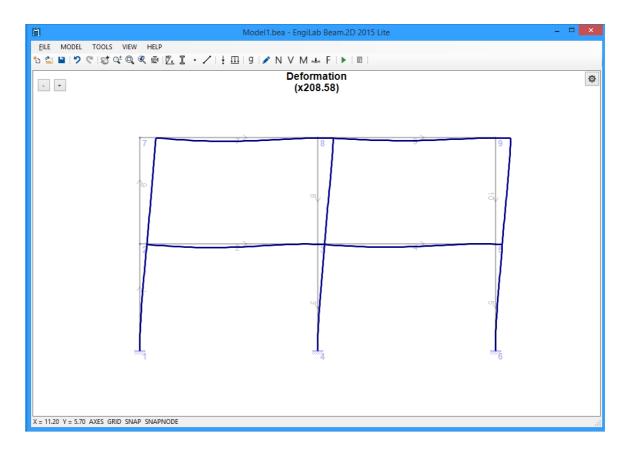
Note that if you move the pointer over an Element, you can read the corresponding value of the Diagram, as shown below for the Bending Moment Diagram case. This happens for all Diagrams and also for the Deformation and the Free Body Diagram.





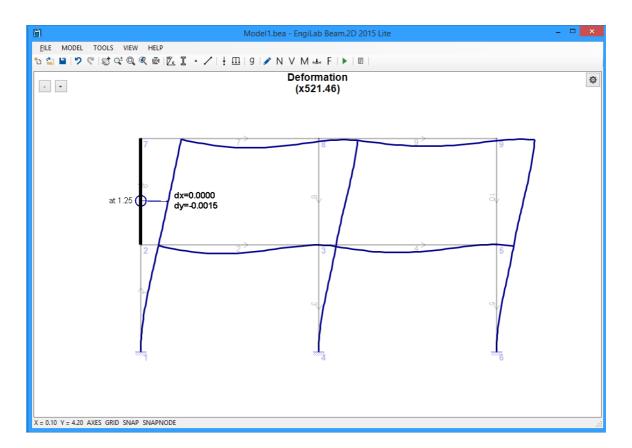
4. Deformation

Click $\stackrel{1}{\rightarrow}$, to see the Model Deformation. If the Deformation is out of Scale, **Click the "Zoom All" button** to automatically scale it. You should take the following picture. The program reports also the Deformation magnification, in our example x208.58.





You can adjust scaling yourself by using the +/- buttons at the top left of the picture. If you click the "+" button a few times, you may get a picture like the one below where the magnification factor is now x521.46.



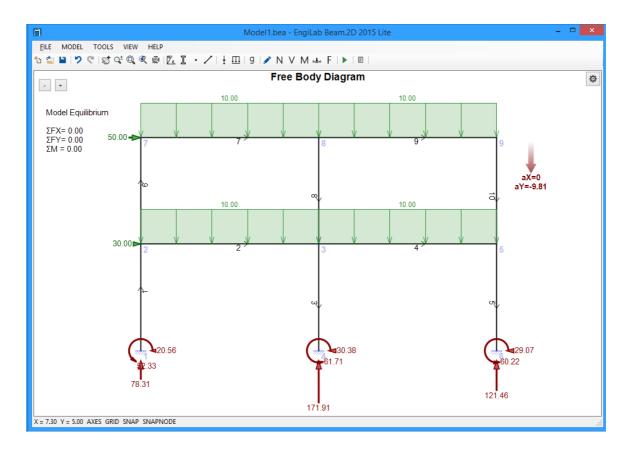
See above that you can get the deformation values on screen, if the mouse pointer moves over an element.

IMPORTANT: The values that are given on screen are the x and y displacements of the corresponding point of each element **in Local Element Axes**. For example, in the picture below Element 6 goes upward, which means that the Element x-Axis is pointing upwards and the Element y-Axis is pointing to the left. The point at 1.25 from the Element start (Node 2) has a deformation dy=-0.0015 m towards the Element y-direction (that means 0.0015 m to the right of the picture), which is equivalent to a deformation DX=0.0015 m in Global Axes (Global X-Axis points towards the right of the picture).



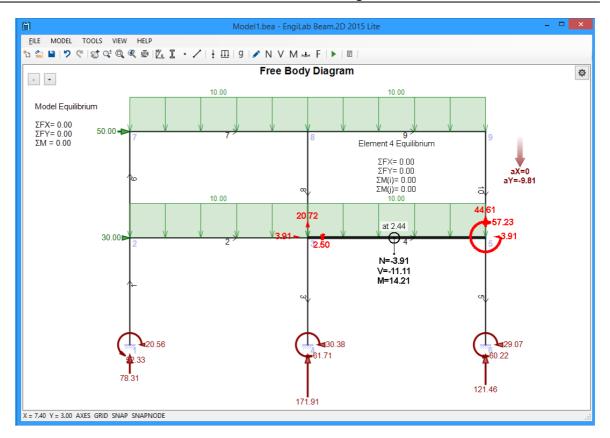
5. Free Body Diagram (F)

Click F, to see the Free Body Diagram of the Model. The Free Body Diagram shows the support reactions on screen and also the calculations of the equilibrium of the Model.



Note that if you move the pointer over an Element, you can read the corresponding N, V, M values, as shown below. The Element End Forces are also given on screen, for the specific Element and also the calculation of the equilibrium of the specific Element.





IMPORTANT: In the example above, we see the calculations of the equilibrium of Element 4 of the Model. All equilibrium calculations (Σ FX, Σ FY, Σ M(i), Σ M(j)) are zero, otherwise there would be a problem in our Model or in the analysis results. One may want to calculate the equilibrium of Element 4 on his own, for example the value Σ FY. There is an external uniform load with value 10, acting along Element 4 (Length = 5 m), which gives an external load of 10*5=50 kN acting towards -Y direction (downwards). The Element end forces sum up to 20.72+44.61=65.33 kN, towards Y direction (upwards). So why is there this difference of 65.33-50 = 15.33 kN? Is there a problem with the calculations?

The answer is NO. This is because of the self-weight of Element 4 which results to an additional uniform load (which is not shown on screen) acting towards -Y direction. Let's calculate this additional load:

Self weight of Element 5 (in kN) = Mass * Acceleration = Volume * Density * Acceleration = Area * Length * Density * Acceleration = 0.125 * 5 * 2.5 * 9.81 = 15.33 kN.

So, the correct equilibrium calculation should be: $\Sigma FY = 50 + 15.33 - 65.33 = 0.00$ kN which is reported by the program for Element 4.



7.1.13 Step 12. View the analytical results



Click \blacksquare to see the analytical results.

1. Node Displacements

			actions Full Report (RTF	Cell color:	Decimal places:
Node ID	X-Displacement	Y-Displacement	Z-Rotation	Free	4 📮 Displacer
1	0.0000	0.0000	0.0000	Constrained	4 + Rotation
2	0.0009	0.0000	-0.0004	Spring	
3	0.0009	-0.0001	-0.0003	- pany	2 🛓 Force
4	0.0000	0.0000	0.0000	Positive: Global axes	2 🚔 Moment
5	0.0009	0.0000	-0.0003	Global axes	
6	0.0000	0.0000	0.0000	+	
7	0.0022	0.0000	-0.0004		
8	0.0021	-0.0001	-0.0002	Reset	
9	0.0021	-0.0001	-0.0002		
				0	



lode Displacer	ments Eleme	nt End Forces	Support Reacti	ions Full Report	(RTF) Analy	sis Validation	_
Element ID	Node ID	Axial Force	Shear Force	Bending Moment		Cell color:	Decimal places
1 Start	1	-78.31	20.56	-52.33		Hinge	· · · · · · · · · · · · · · · · · · ·
1 End	2	-59.91	20.56	9.34			•
2 Start	2	-9.45	19.40	7.55		Sign convention:	2 🛉 Force
2 End	3	-9.45	-45.93	-58.79		Diagrams	2 🔶 Mom
3 Start	3	-153.52	30.38	-29.43			
3 End	4	-171.91	30.38	61.71		↓	
4 Start	3	-3.91	20.72	2.50			
4 End	5	-3.91	-44.61	-57.23		⁺L≻∐ ∔₊	
5 Start	5	-103.06	29.07	-26.98			
5 End	6	-121.46	29.07	60.22			
6 Start	2	-40.52	0.01	1.79		0	
6 End	7	-22.12	0.01	1.81			
7 Start	7	-49.99	22.12	1.81			
7 End	8	-49.99	-43.20	-50.89			
8 Start	8	-68.47	24.83	-42.64			
8 End	3	-86.87	24.83	31.86			
9 Start	8	-25.16	25.27	-8.25			
9 End	9	-25.16	-40.06	-45.22			
10 Start	9	-40.06	25.16	-45.22			
10 End	5	-58.45	25.16	30.25			ОК

2. Element Forces

3. Support Reactions

Node Displa	acements	Element End Forces	Support Reactions	Full Report (RTF)	Analysis Validation	ı	
Node ID 1 4 6	X-Force -20.56 -30.38 -29.07	e Y-For 78.31 171.91 121.46	52.3	1		Cell color: Free Constrained Spring Positive: Slobal axes Constrained Slobal axes Constrained Constrained Constrained Spring Constrained Constrained Constrained Spring Constrained Con	Decimal places: 4 Displaces 4 Rotation 2 Moment
							ОК



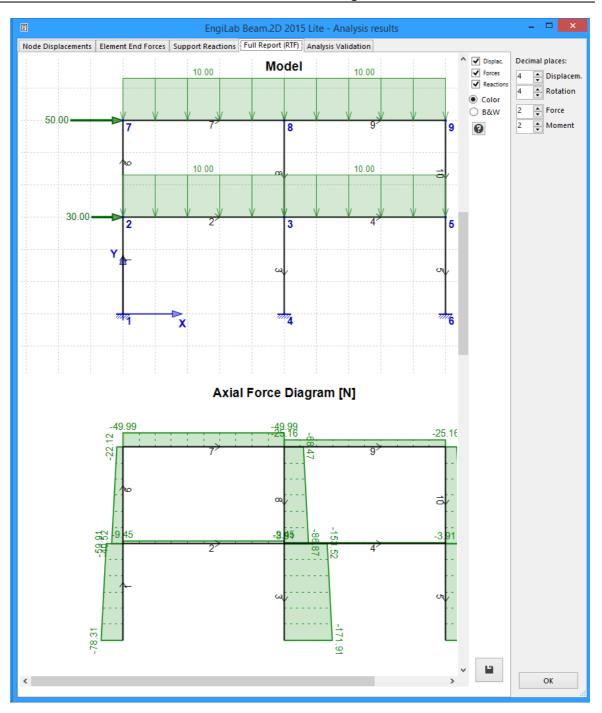


4. Full Analysis Report (RTF)

		EngiLab Bear	m.2D 2015 Lite - Analysis re	sults	
Node Displacements Ele	nent End Forces Supr	ort Reactions Full F	Report (RTF) Analysis Validation		
ngiLab Beam.2D 201				▲ ✓ Displ	lac. Decimal places:
Nodel: "Model1.bea"				(comp	
Report created: Sunday	10 May 2015 21:33	}∙17		Force	4 Displace
iopont oroatou, ounday				✓ Read	ctions 4 📮 Rotatio
lode Displacements				Colo	
				O B&V	W 2 📮 Force
Node ID X-	Displacement Y-D	isplacement	Z-Rotation	<u> </u>	
1	.0000	0.0000	0.0000	•	2 📥 Momen
2	0.0009	0.0000	-0.0004		
3	0.0009	-0.0001	-0.0003		
4	0.0000	0.0000	0.0000		
5	0.0009	0.0000	-0.0003		
6	0.0000	0.0000	0.0000		
7	0.0022	0.0000	-0.0004		
8	0.0021	-0.0001	-0.0002		
9	0.0021	-0.0001	-0.0002		
ign convention: Positive ac	cording to Global Axes				
isplacements reported with					
otations (in Radians) report	ed with 4 decimal places				
ackground color: Free D)F Constrained DOF	Spring DOF			
lement End Forces					
Classet TD	Audal Causa	chara Causa Da	dian Namat		
Element ID 1 Start	Axial Force -78.31	Shear Force Be 20.56	-52.33		
1 End	-59.91	20.56	9.34		
2 Start	-9.45	19.40	7.55		
2 End	-9.45	-45.93	-58.79		
3 Start	-153.52	30.38	-29.43		
3 End	-171.91	30.38	61.71		
4 Start	-3.91	20.72	2.50		
4 End	-3.91	-44.61	-57.23		
5 Start	-103.06	29.07	-26.98		
5 End	-121.46	29.07	60.22		
6 Start	-40.52	0.01	1.79		
6 End	-22.12	0.01	1.81		
7 Start	-49.99	22.12	1.81		
7 End	-49.99	-43.20	-50.89		
8 Start 8 End	-68.47 -86.87	24.83 24.83	-42.64		
8 End 9 Start	-25.16	24.85	31.86 -8.25		
9 End	-25.16	-40.06	-45.22		
10 Start	-40.06	25.16	-45.22		
10 End	-58.45	25.16	30.25		
gn convention: According Axial force: Positive when Shear force: Positive when Bending moment: Positive broces reported with 2 deci ending moments reported v ackground color: Normal	member is in tension n it rotates the member cl when the "bottom" fiber is nal places with 2 decimal places	ockwise	in compression)		
Support Reactions Node ID	X-Force	Y-Force	Z-Moment		
1	-20.56	78.31	52.33		
	-30.38	171.91	61.71		
4	-29.07	121.46	60.22		
4 6					
6 gn convention: Positive ac	nal places			× 🗎	ОК

The Full Analysis Reports includes also the pictures of the Model, N, V, M Diagrams, Deformation and Free Body Diagram, as shown below.







5. Analysis Validation

5a. Global (Model) Equilibrium

	EngiLab Beam.2	2D 2015 Lite - /	Analysis results		- 🗆 🗙
Node Displacements Element End Global (Model) Equilibrium Individ Load/Reaction type Individ Nodal Loads Elemental Loads Acceleration (Body) Loads Support Reactions SUM SUM	Forces Support R dual Element Equili Σ(FX) 80.00 0.00 -80.00 0.00 0.00		Σ(MZ at 0,0) -390.00 -1000.00 -858.38 2248.38 0.00	Positive: Global axes	Decimal places: 4 Displacem. 4 Rotation 2 Force 2 Moment
					ОК

5b. Individual Element Equilibrium

lode Displace	ments Eleme	nt End Fo	rces Suppor	t Reactions Fu	ull Report (F	(TF) Analysis	Validation							
Global (Mode	l) Equilibrium	Individua	al Element Eq	uilibrium									Decimal place	:es:
Element ID	FX Elemental Force	FX Body Force	FX End Force i	FX End Force j	SUM ΣFX	FY Elemental Force	FY Body Force	FY End Force i	FY End Force j	SUM ΣFY	SUM ΣMZ at i	SUM ΣMZ at j	4 🔹 Disp 4 🔹 Rota	
▶ 1	0.00	0.00	-20.56	20.56	0.00	0.00	-18.39	78.31	-59.91	0.00	0.00	0.00	2 🌲 Fore	ce
2	0.00	0.00	9.45	-9.45	0.00	-50.00	-15.33	19.40	45.93	0.00	0.00	0.00	2 🌲 Mor	ment
3	0.00	0.00	30.38	-30.38	0.00	0.00	-18.39	-153.52	171.91	0.00	0.00	0.00		
4	0.00	0.00	3.91	-3.91	0.00	-50.00	-15.33	20.72	44.61	0.00	0.00	0.00		
5	0.00	0.00	29.07	-29.07	0.00	0.00	-18.39	-103.06	121.46	0.00	0.00	0.00		
6	0.00	0.00	-0.01	0.01	0.00	0.00	-18.39	40.52	-22.12	0.00	0.00	0.00		
7	0.00	0.00	49.99	-49.99	0.00	-50.00	-15.33	22.12	43.20	0.00	0.00	0.00		
8	0.00	0.00	24.83	-24.83	0.00	0.00	-18.39	-68.47	86.87	0.00	0.00	0.00		
9	0.00	0.00	25.16	-25.16	0.00	-50.00	-15.33	25.27	40.06	0.00	0.00	0.00		
10	0.00	0.00	25.16	-25.16	0.00	0.00	-18.39	-40.06	58.45	0.00	0.00	0.00	ОК	



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Chapter



License Agreement

8 License Agreement

- <u>License Agreement (Lite Edition)</u>
- License Agreement (Pro Edition)

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